

Appendix G
Traffic Study

**City of Lake Forest
SPORTS PARK AND RECREATION CENTER**

Traffic Study

March 2010



Draft

City of Lake Forest

SPORTS PARK AND RECREATION CENTER

Traffic Study

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March 11, 2010

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**City of Lake Forest
SPORTS PARK AND RECREATION CENTER
Traffic Study**

INTRODUCTION

This report presents the results of a traffic analysis performed for the proposed Sports Park and Recreation Center project on the southwest corner of Portola Parkway and future extension of Rancho Parkway in the City of Lake Forest. The report addresses the traffic impacts related to the proposed project, and has been prepared for the City of Lake Forest in support of the project Environmental Impact Report (EIR) for this project. The report contains documentation of the methodology and assumptions used in the analysis, and presents the results and findings of the traffic impacts of the proposed project.

PROJECT DESCRIPTION

Figure 1 shows the study area and the project location in relation to the surrounding circulation system. Located in the northeastern portion of the City of Lake Forest, the proposed project illustrated in Figure 2 is adjacent to the intersection of Portola Parkway and future Rancho Parkway on the southwest corner and involves three parcels currently owned by Rados, Baker and the County of Orange. The City of Lake Forest proposes to acquire these properties for development of the Sports Park project. The proposed project being analyzed in this report is the development of an approximate 67.1-acre sports park facility. Based on the Consensus Master Plan (July 2009), the proposed project will encompass a total of 87.7 acres with 67.1 acres in active use and 20.6 acres in a passive use easement. The proposed recreational facilities include lighted sports facilities with baseball, soccer and multi-purpose fields, and basketball and/or tennis courts, a community center including a gymnasium as well as meeting/multi-purpose rooms, and children's playgrounds.

As can be seen in Figure 2, access to the sports park and recreation center complex is proposed via four driveways. These driveways are assumed to be phased in the following order as the related property becomes available. Driveway 1A is on Portola Parkway, a six lane major arterial, just south of the intersection of Portola Parkway and Rancho Parkway, Driveway 1B is at the current terminus of Vista Terrace, a two-lane local road, and Driveways 2 and 3 are located along the future extension of Rancho Parkway, a four-lane primary arterial, just west of the intersection of Portola Parkway and Rancho Parkway.

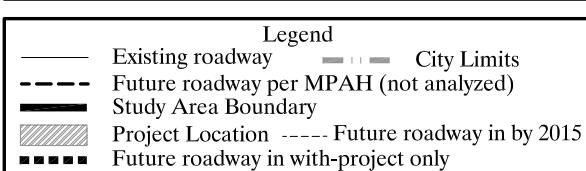
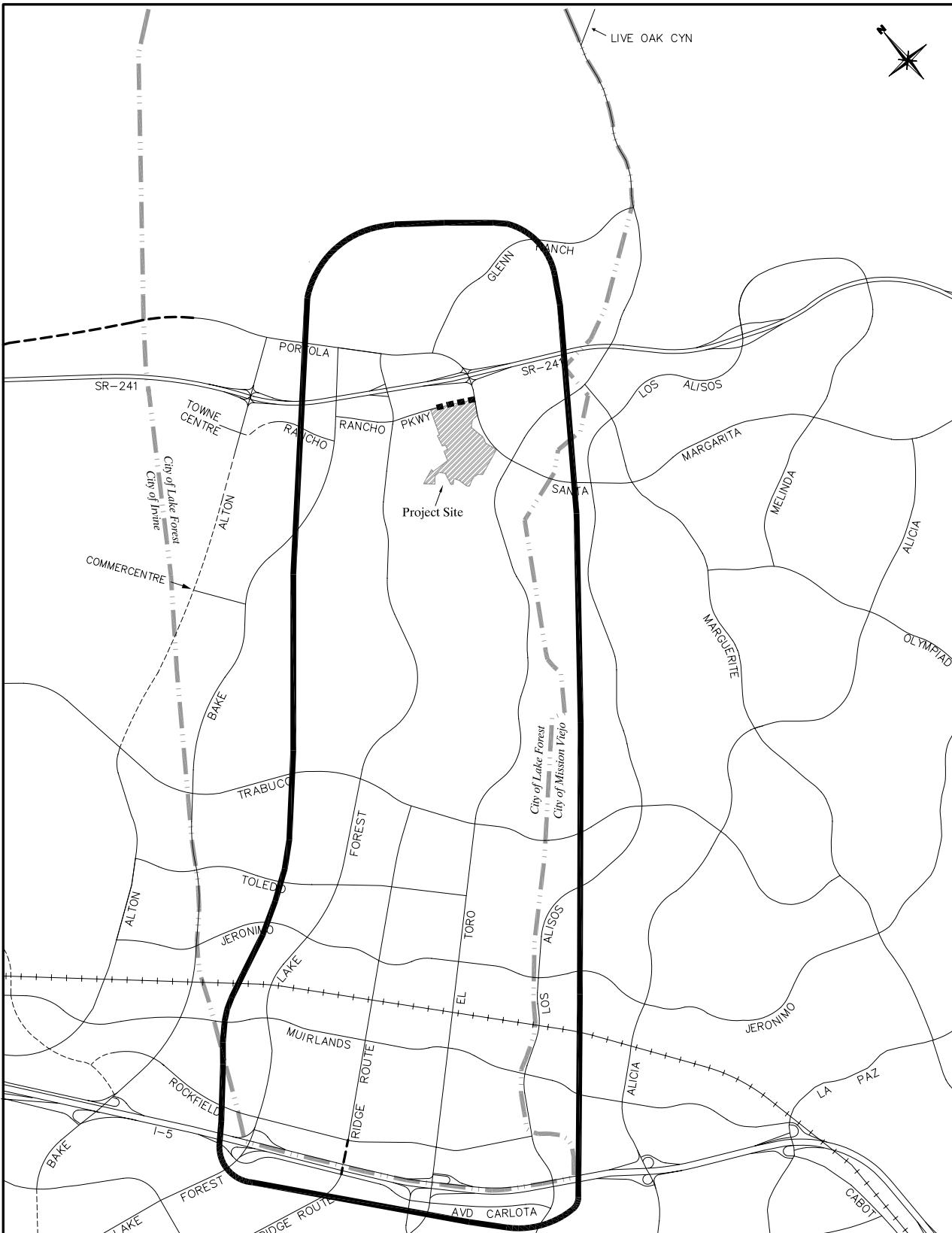


Figure 1

PROJECT LOCATION



Legend



Access points

Figure 2

PROJECT SITE

The proposed project is anticipated to occur in two to three phases as property becomes available. However, for purposes of this analysis, full buildup of the proposed sports park and recreation center facilities is assumed to occur in one phase with the project opening during 2011 which is referred to in this report as short-term conditions. This is considered a worst-case analysis because it would place all the project trips on the current roadway network without the benefit of any future improvements particularly those that will be funded by the Lake Forest Transportation Mitigation Program (LFTM). An analysis in which other growth is assumed as well as roadway improvements will be conducted for short-term conditions (referred to in this report as year 2015 cumulative analysis).

Table 1 summarizes the land use and trip generation for no-project, which are existing land uses, and with-project conditions for the project site under short-term conditions. Table 1 also shows that the existing passive use park and portion of the mining/utility use south of the future extension of Rancho Parkway are replaced by the proposed project. The trip rates for the sports park are based on case studies of similar parks conducted throughout Orange County and Los Angeles County and assume that all facilities/park amenities (i.e., community/recreation center and all fields) are in concurrent use.

The proposed project includes 20.6 acres of passive use easement and the 67.1 sports park and recreation center that encompasses 13 acres of the Rados parcel, 16.1 acres of the Baker parcel and 38 acres of the County parcel and generates 3,642 average daily trips (ADT).

PROJECT TRIP DISTRIBUTION

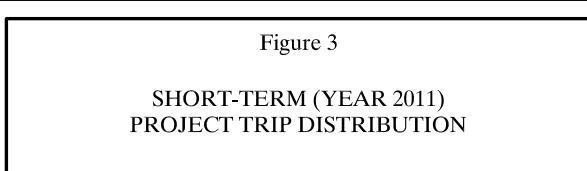
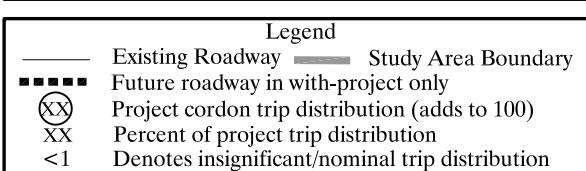
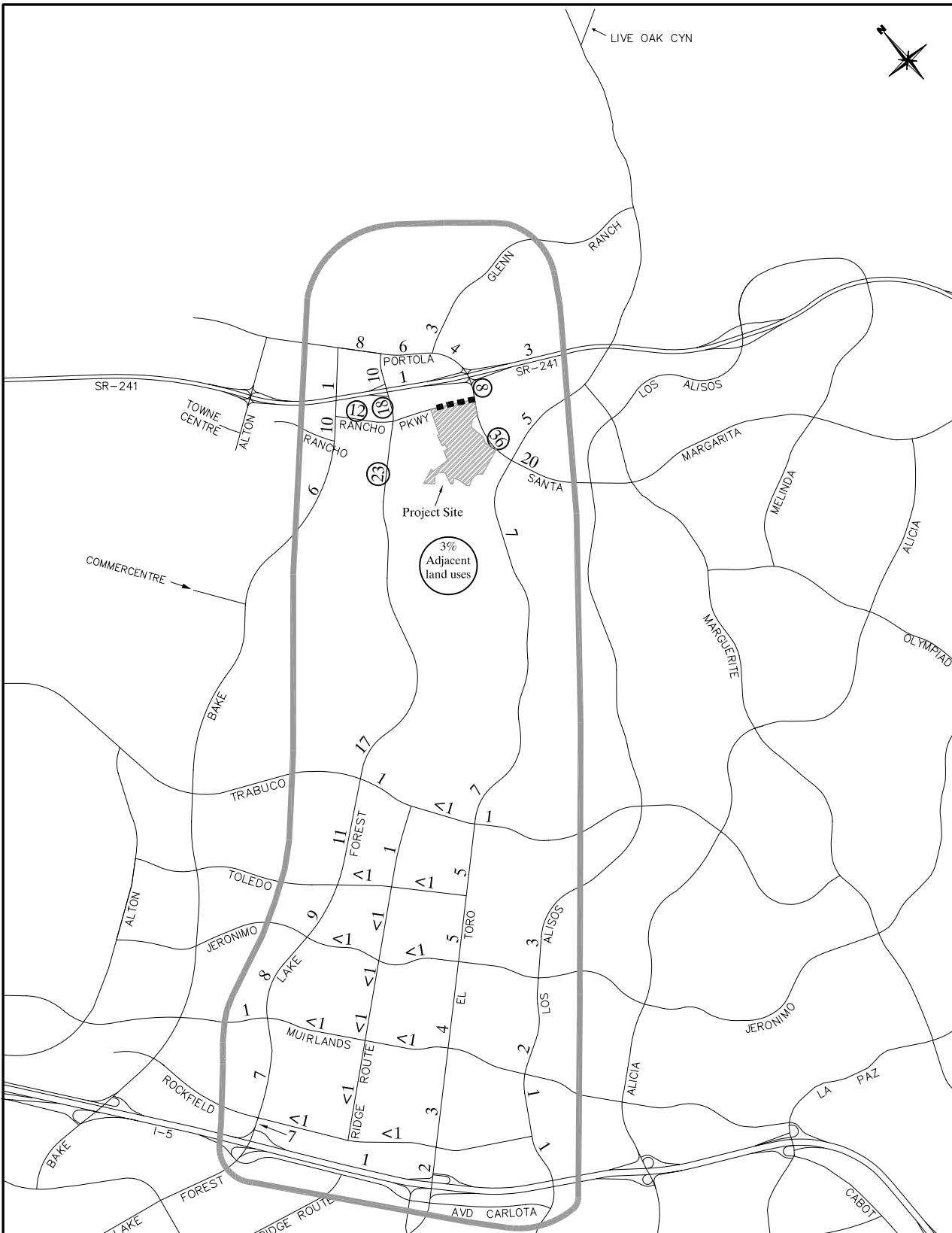
Trip distribution patterns for the proposed project land uses are presented in Figures 3 and 4 for the two short-term conditions analyzed in this report (year 2011 project opening and year 2015 cumulative) which are based on the City of Lake Forest's traffic model's distribution of daily project traffic. These percentages differ slightly in the peak hours, and the traffic model uses the individual peak hour distribution patterns to assign peak hour trips.

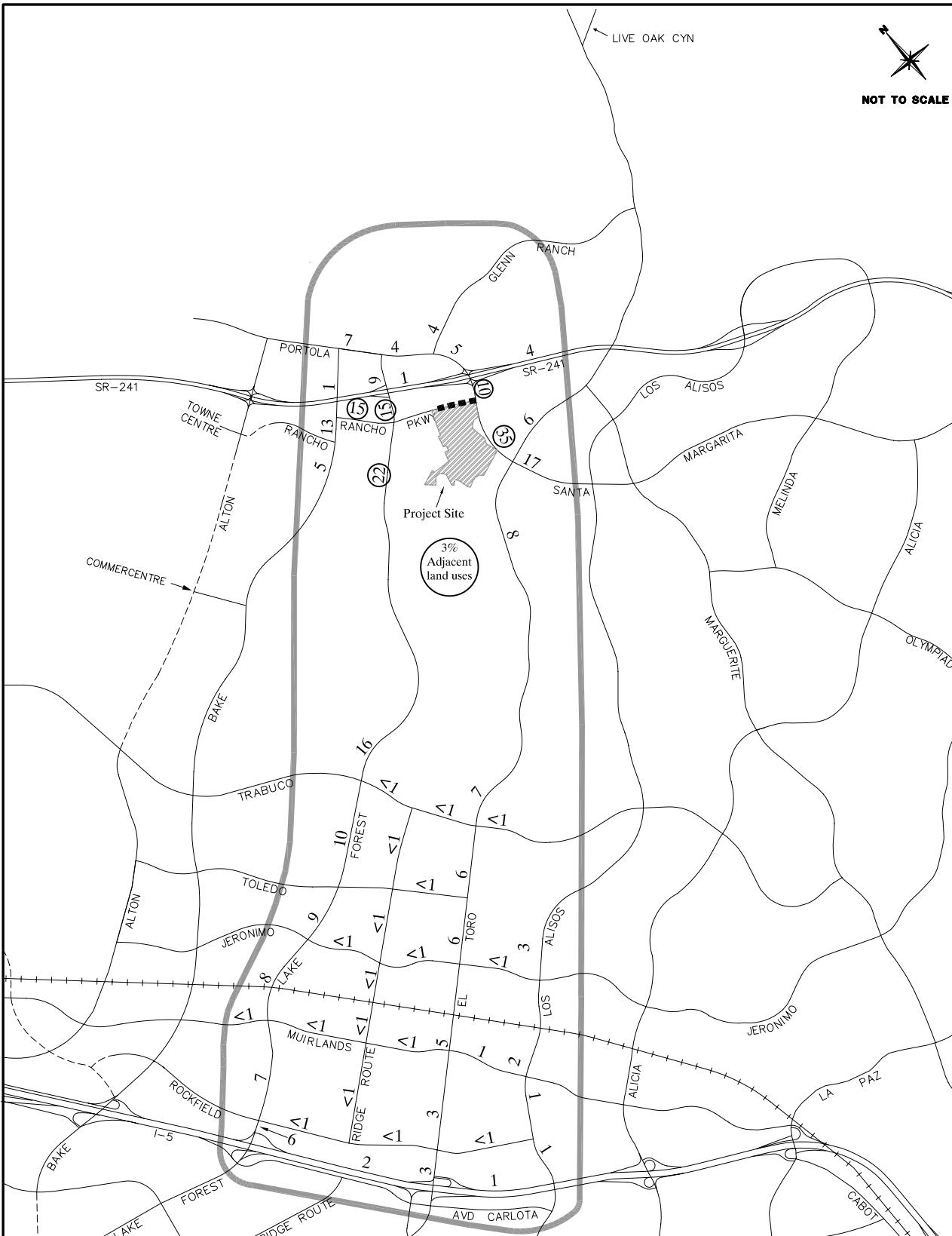
The study area used for the analysis was determined from the distribution of project trips on the adjacent circulation system and impact criteria guidelines defined in this report. It is based on where project traffic dissipates to less than significant levels (i.e., change in intersection capacity utilization (ICU) of less than .03). Generally these thresholds are met when the project trip distribution is below 10 percent. For example, the intersection of Bake Parkway and Rancho Parkway South carries a significant amount of project traffic, but beyond that location, project traffic dissipates to less than a significant amount.

Table 1

SHORT-TERM CONDITIONS (YEAR 2011 AND YEAR 2015 CUMULATIVE) PROJECT SITE
LAND USE AND TRIP GENERATION SUMMARY

Land Use	Parcel	Units	AM Peak Hour			PM Peak Hour			ADT
			In	Out	Total	In	Out	Total	
No-Project									
Vacant	Rados	13 Acre	0	0	0	0	0	0	0
Mining/Utility	Baker	16.1 Acre	25	15	40	9	12	21	401
Park (a)	County	58.6 Acre	1	0	1	1	1	2	93
Total		87.7 Acre	26	15	41	10	13	23	494
With-Project									
Sports Park	Rados	13 Acre	0	0	0	44	53	97	699
Sports Park	Baker	16.1 Acre	0	0	0	55	66	121	866
Park (b)	County	20.6 Acre	0	0	0	0	0	0	33
Sports Park	County	38 Acre	0	0	0	129	156	285	2,077
Total With-Project		87.7 Acre	0	0	0	228	275	503	3,642
Difference			-26	-15	-41	218	262	480	3,148
Trip Rates									
Park		Acre	.01	.00	.01	.02	.02	.04	1.59
Sports Park		Acre	.01	.00	.01	3.40	4.10	7.50	53.80
Mining/Utility		Acre	1.57	.92	2.49	.59	.73	1.32	24.9
Vacant		Acre	.00	.00	.00	.00	.00	.00	.00
(a) Currently a passive use park.									
(b) Passive use easement.									
Abbreviation: ADT – average daily trips									





Legend

- Existing Roadway
- Study Area Boundary
- - - Future Roadway in by 2015
- Project roadway in with-project only
- XX Project cordon trip distribution (adds to 100)
- <1 Percent of project trip distribution
- <1 Denotes insignificant/nominal trip distribution

Figure 4

SHORT-TERM (YEAR 2015 CUMULATIVE)
PROJECT TRIP DISTRIBUTION

ANALYSIS SCOPE AND TRAFFIC FORECASTING METHODOLOGY

The proposed project is analyzed under opening year conditions referred to as short-term conditions in this report (expected in year 2011) and cumulative conditions (year 2015). A comparison is made of the levels of service of with-project conditions to no-project, which has existing land uses (i.e., mining/utility and park uses) assumed on the project site. The extension of Rancho Parkway to Portola Parkway is assumed with the proposed project and is referred to as a project feature.

Average daily traffic (ADT) and peak hour volumes on the circulation system for existing conditions in the project area are first identified, and then the ADT and peak hour forecasts for short-term conditions (year 2011 and year 2015 cumulative) are presented. Existing ADT and peak hour counts were conducted in 2008. Short-term year 2011 forecast volumes used in the analysis were derived using two steps. First, a growth factor of two (2) percent per year for a total of six (6) percent was applied to the existing 2008 counts to derive year 2011 no-project conditions. Then the City's Lake Forest Traffic Analysis Model (LFTAM) was used to derive the change in traffic due to the proposed project land uses including the addition of the Rancho Parkway extension to Portola Parkway as a project feature. The circulation system for year 2011 in other parts of the study area assumes existing conditions.

In the cumulative analysis year 2015, Alton Parkway is assumed to be connected between Towne Centre Drive and Irvine Boulevard. Also, a linear growth of traffic and development is assumed between now and year 2030, therefore a growth of 25 percent in the Opportunities Study Area (OSA) is assumed for cumulative analysis purposes including the current General Plan land use assumptions in the Shea/Baker area. The proposed project is assumed to be built out under with-project conditions for worst-case analysis purposes.

Mitigation measures needed to reduce project impacts to a level of insignificance are identified if necessary for year 2011 and/or year 2015 cumulative. Should the project require mitigation measures, reference will be first made of any LFTM improvements because the proposed project is part of the OSA Program.

The LFTAM is derived from the Orange County Transportation Analysis Model (OCTAM) which is maintained by the Orange County Transportation Authority (OCTA). The LFTAM was developed according to the Orange County sub-area traffic modeling guidelines that have been adopted

by the OCTA, and the OCTA has certified the traffic model as being consistent with the OCTAM regional model. OCTA recognizes the role of the sub-area models for local roadways, and purpose of their certification is to enable the sub-area models to provide that type of information, but with an overall consistent regional context.

For descriptive purposes, the modeling processes in the LFTAM can be divided into the following three general components:

1. Trip Generation
2. Trip Distribution/Mode Choice
3. Traffic Assignment

In the trip generation component of the traffic model, the amount of vehicle traffic generated by existing and future land use development is estimated. In the LFTAM, land use data is defined according to specific land use categories. The information is quantified by traffic analysis zones (TAZs) that have been defined in the City of Lake Forest as well as throughout the remainder of the study model area. For trip generation purposes, land use data is typically comprised of detailed information by acreage or floor area for non-residential uses and number of dwelling units by density classification for residential uses. As part of the modeling process, the land use data is converted to socioeconomic categories such as dwelling units, population, employment, workers per household and income. The socioeconomic categories applied in the traffic model are the same categories that are applied in the OCTAM regional model. Vehicle trip generation estimates for the LFTAM are produced using socioeconomic trip generation rates that yield similar trip generation to land use based trip generation rates.

In the trip distribution/mode choice component of the traffic model, vehicle trip generation estimates are distributed using regional travel forecast data from the OCTAM model, thereby incorporating regional trip distribution patterns into the LFTAM. The regional traffic data is obtained from the OCTAM regional model in the form of vehicle trips, and hence also incorporates mode choice relationships (i.e., vehicle occupancy, transit trips, etc.) established in the OCTAM regional model. The resulting vehicle trip patterns are converted to actual traffic volumes on the roadway system in the traffic assignment component of the LFTAM. The traffic assignment component applies procedures that are sensitive to the capacity of the circulation system network and give forecast peak hour (AM and PM) volumes as well as ADT traffic volumes on that network.

ANALYSIS OF PEAK PERIODS

The peak of the use for the project site would usually occur on Saturday and the analysis in this report is for an average weekday in which peak conditions of the adjacent street are analyzed. To address a comment made during the Notice of Preparation (NOP) review period by the City of Mission Viejo regarding impacts within their City, peak hour traffic counts were taken on this roadway segment on an average weekday and weekend days (from Thursday to Tuesday) to determine if the peak is different than the average weekday peak (see Table 2 for the results and Appendix B for actual count sheets).

Table 2 Santa Margarita Parkway just east of El Toro Road		
Daily Count	Day	Date (2009)
33,178	Thursday/Tuesday *	July 30/August 4
34,412	Friday	July 31
28,025	Saturday	August 1
24,894	Sunday	August 2
30,570	Monday	August 3

* Partial day of counts that were combined.

Figure 5 shows a comparison of the counts that were taken, on a combination Thursday/Tuesday (partial counts were taken on these days), and Friday through Monday. As can be seen from this figure, during a typical weekday on this road, two peak periods (8:00 AM – 10:00 AM and 5:00 PM – 7:00 PM) occur and one peak period in a typical weekend day (in this case, Saturday, between 12:00 PM and 3:00 PM). The two weekday peak periods present the worst case over the one peak period in the highest weekend day (Saturday). In addition, the combination of the weekday background traffic with project traffic results in higher traffic volumes than the combination of weekend (i.e., Saturday) background traffic with project traffic. Therefore the forecasts presented in this report are based on the worst case.

LEVEL OF SERVICE AND PROJECT MITIGATION CRITERIA

In this report, a set of performance criteria is utilized to identify future level of service (LOS) deficiencies on the study area circulation system and also to define impacts and peak hour Intersection Capacity Utilization (ICU) values of significance. Traffic LOS is designated “A” through “F” with LOS “A” representing free flow conditions and LOS “F” representing severe traffic congestion. Tables 3 and 4 summarize the general LOS descriptions for intersections and freeways/tollways, respectively.

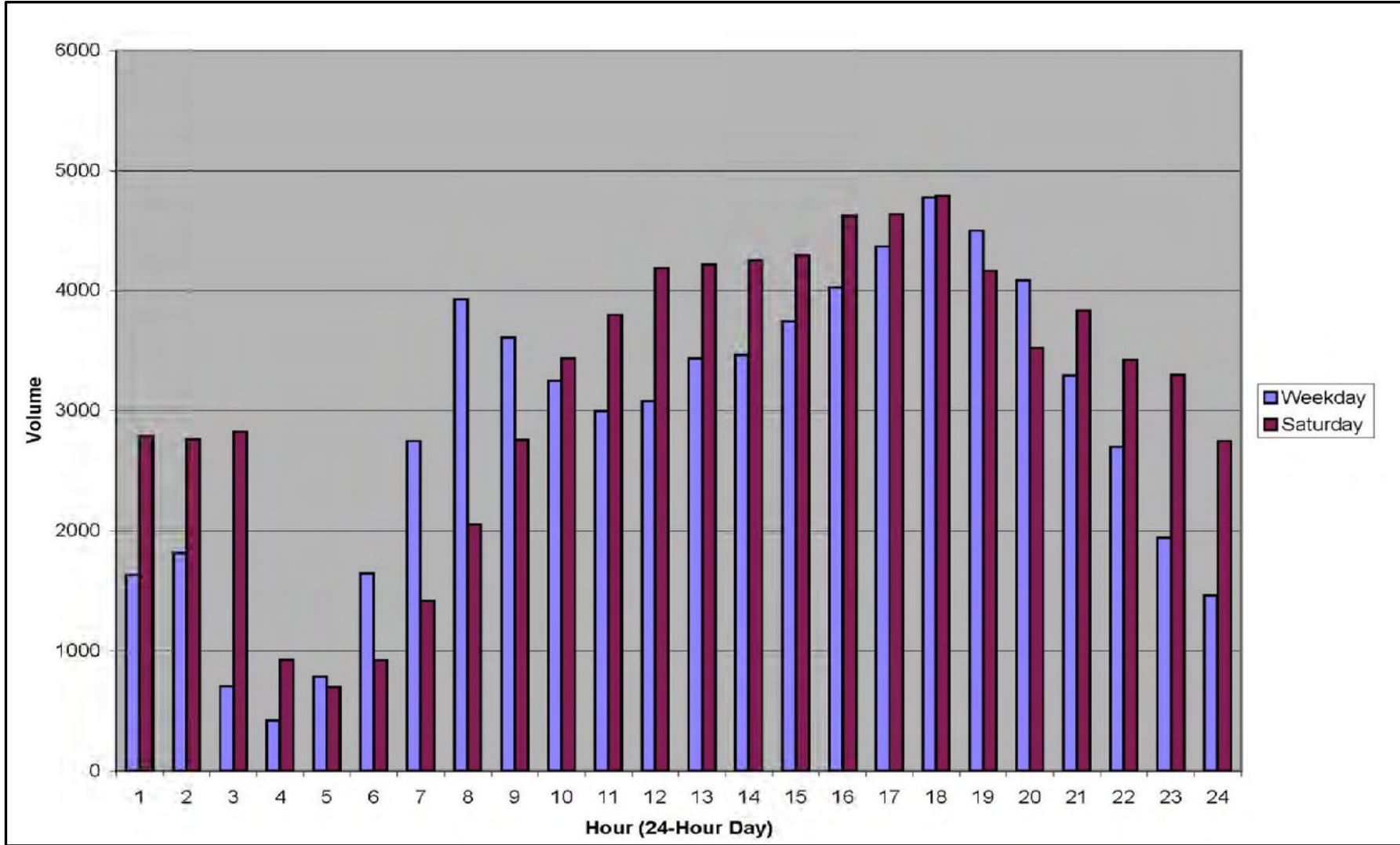


Figure 5

SANTA MARGARITA PARKWAY JUST SOUTH OF
EL TORO ROAD 24-HOUR COUNTS

Table 3
LEVEL OF SERVICE DESCRIPTIONS – SIGNALIZED INTERSECTIONS

Levels of service (LOS) for signalized intersections are defined in terms of either average control delay that is measured in seconds (HCM methodology) or intersection capacity utilization (ICU) values as follows:

LOS	Description	Average Delay (sec)¹	ICU²
A	LOS “A” describes operations with low control delay, up to 10 seconds per vehicle. This LOS occurs when progression is extremely favorable and most vehicles arrive during the green phase. Many vehicles do not stop at all. Short cycle lengths may tend to contribute to low delay values.	≤ 10.0	≤ .60
B	LOS “B” describes operations with control delay greater than 10 and up to 20 seconds per vehicle. This level generally occurs with good progression, short cycle lengths, or both. More vehicles stop than the LOS “A,” causing higher levels of delay.	10.1 – 20.0	.61 - .70
C	LOS “C” describes operations with control delay greater than 20 and up to 35 seconds per vehicle. These higher delays may result from only fair progression, longer cycle lengths, or both. Individual cycle failures may begin to appear at this level. Cycle failure occurs when a given green phase does not serve queued vehicles, and overflows occur. The number of vehicles stopping is significant at this level, though many still pass through the intersection without stopping.	20.1 – 35.0	.71 - .80
D	LOS “D” describes operations with control delay greater than 35 and up to 55 seconds per vehicle. At LOS “D,” the influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, and high V/C ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.	35.1 – 55.0	.81 - .90
E	LOS “E” describes operations with control delay greater than 55 and up to 80 seconds per vehicle. These high delay values generally indicate poor progression, long cycle lengths, and high V/C ratios. Individual cycle failures are frequent.	55.1 – 80.0	.91 – 1.00
F	LOS “F” describes operations with control delay in excess of 80 seconds per vehicle. This level, considered unacceptable to most drivers, often occurs with oversaturation, that is, when arrival flow rates exceed the capacity of lane groups. It may also occur at high V/C ratios with many individual cycle failures. Poor progression and long cycle lengths may also contribute significantly to high delay levels.	> 80.0	> 1.00

¹ Source: *Highway Capacity Manual 2000 (HCM 2000)*, Transportation Research Board, National Research Council.
² Source: Orange County Congestion Management Program (CMP).

Table 4
LEVEL OF SERVICE DESCRIPTIONS – FREEWAYS/TOLLWAYS

LOS	Description	Volume/ Capacity¹
A	LOS “A” describes free-flow operations. Free-flow speeds (FFS) prevail. Vehicles are almost completely unimpeded in their ability to maneuver with the traffic stream. The effects of incidents or point breakdowns are easily absorbed at this level.	.00 – .30
B	LOS “B” represents reasonably free-flow, and FFS are maintained. The ability to maneuver with the traffic stream is only slightly restricted, and the general level of physical and psychological comfort provided to drivers is still high. The effects of minor incidents and point breakdowns are still easily absorbed.	.31 – .50
C	LOS “C” provides for flow with speeds at or near the FFS of the freeway/tollway. Freedom to maneuver within the traffic stream is noticeably restricted, and lane changes require more care and vigilance on the part of the driver. Minor incidents may still be absorbed, but the local deterioration in service will be substantial. Queues may be expected to form behind any significant blockage.	.51 – .71
D	LOS “D” is the level at which speeds begin to decline slightly with increasing flows and density begins to increase somewhat more quickly. Freedom to maneuver within the traffic stream is more noticeably limited, and the driver experiences reduced physical and psychological comfort levels. Even minor incidents can be expected to create queuing, because the traffic stream has little space to absorb disruptions.	.72 – .89
E	At its highest density value, LOS “E” describes operation at capacity. Operations at this level are volatile, because there are virtually no usable gaps in the traffic stream. Vehicles are closely spaces, leaving little room to maneuver with the traffic stream at speeds that still exceed 49 miles per hour. Any disruption of the traffic stream, such as vehicles entering from a ramp or a vehicle changing lanes, can establish a disruption wave that propagates throughout the upstream traffic flow. At capacity, the traffic stream has no ability to dissipate even the most minor disruption, and any incident can be expected to produce a serious breakdown with extensive queuing. Maneuverability with the traffic stream is extremely limited, and the level of physical and psychological comfort afforded the driver is poor.	.90 – 1.00
F	LOS “F” describes breakdowns in vehicular flow. Such conditions generally exist within queues forming behind breakdown points, and are the result of a bottleneck downstream point. LOS “F” is also used to describe conditions at the point of the breakdown or bottleneck and the queue discharge flow that occurs at speeds lower than the lowest speed for LOS “E,” as well as the operations within the queue that forms upstream. Whenever LOS “F” conditions exist, they have the potential to extend upstream for significant distances.	>1.00

¹ Source: *Highway Capacity Manual 2000 (HCM 2000)*, Transportation Research Board, National Research Council.

The intersection criteria involve the use of peak hour ICU values. The ICU ranges that correspond to LOS “A” through “F” were previously presented in Table 3. By practice, the ICU methodology assumes that intersections are signalized. LOS “D” (ICU not to exceed .90) is the performance standard for the intersections in the study area.

The freeway/tollway mainline and freeway/tollway ramp criteria are based on peak hour V/C ratios (as previously presented in Table 4). The freeway/tollway mainline and ramp capacities applied in this analysis are based on information contained in the Caltrans Highway Design Manual and the Caltrans Ramp Meter Design Manual. LOS “E” (V/C not to exceed 1.00) has been established by Caltrans as the operating standard for freeway/tollway mainline segments and freeway/tollway ramps. This standard is also consistent with the LOS “E” standard specified in the Orange County CMP for CMP facilities (the freeway/tollway system in the study area is included in the CMP roadway network).

The overall performance criteria applied in this study are summarized in Table 5. The criteria include components for intersections, freeway/tollway ramps, and freeway/tollway mainline segments and are based on LOS calculation methodologies and performance standards that have been adopted by the City of Lake Forest and by the OCTA as part of the CMP. The performance criteria applied here is the same as used in previous OSA traffic analyses.

EXISTING CONDITIONS

Existing Circulation System and Average Daily Traffic Volumes

The existing circulation system in the study area is illustrated in Figure 6 together with existing midblock lanes on arterial roadways and the number of existing travel lanes on freeway/tollway mainline segments. Current average daily traffic (ADT) volumes are illustrated in Figure 7. The arterial volumes are from 2008 counts and the volumes on I-5 and SR-241 are 2007 counts published by the California Department of Transportation (Caltrans) for the California State Highway system.

Existing Peak Hour Intersection Levels of Service

Peak hour intersection turn movement counts were collected for the signalized intersection locations shown in Figure 8. The ICU values based on these counts are summarized in Table 6 and illustrated in Figures 9 and 10 for AM and PM peak hours, respectively. The AM and PM peak hour is

Table 5

PERFORMANCE CRITERIA FOR LOCATIONS ANALYZED WITHIN THE STUDY AREA

I. Intersections

V/C Calculation Methodology

Level of service to be based on peak hour intersection capacity utilization (ICU) values calculated using the following assumptions:

Saturation Flow Rate: 1,700 vehicles/hour/lane

Clearance Interval: .05

Right-Turn-On-Red Utilization Factor*: .75

* “De facto” right-turn lane is assumed in the ICU calculation if 19 feet from edge to outside of through-lane exists and parking is prohibited during peak periods.

Performance Standard

Level of Service D (peak hour ICU less than or equal to .90).

Mitigation Requirement

For ICU greater than the acceptable level of service, mitigation of the project contribution is required to bring intersection back to acceptable level of service or to no-project conditions if project contribution is .02 or greater for all intersections in the study area.

II. Freeway/Tollway Ramps

V/C Calculation Methodology

Level of service to be based on peak hour volume/capacity (V/C) ratios calculated using the following capacities:

Metered On-Ramps

A maximum capacity of 900 vehicles per hour (vph) for a one-lane metered on-ramp with only one mixed-flow lane at the meter.

A maximum capacity of 1,080 (20 percent greater than 900) vph for a one-lane metered on-ramp with one mixed-flow lane at the meter plus one high occupancy vehicle (HOV) preferential lane at the meter.

A maximum capacity of 1,500 vph for a one-lane metered on-ramp with two mixed-flow lanes at the meter.

A maximum capacity of 1,800 vph for a two-lane metered on-ramp with two mixed-flow lanes at the meter.

(continued)

Table 5 (cont)

PERFORMANCE CRITERIA FOR LOCATIONS ANALYZED WITHIN THE STUDY AREA

II. Freeway/Tollway Ramps (cont)

V/C Calculation Methodology (cont)

Toll Ramps (On-Ramps and Off-Ramps)

A maximum capacity of 1,500 vph for a one-lane toll ramp with one cash (stopped) lane and one FasTrak (unstopped) lane.

Non-Metered and Non-Tolled On-Ramps and Off-Ramps

A maximum capacity of 1,500 vph for a one-lane ramp.

A maximum capacity of 2,250 (50 percent greater than 1,500) vph for a two-lane on-ramp that tapers to one merge lane at or beyond the freeway mainline gore point and for a two-lane off-ramp with only one auxiliary lane.

A maximum capacity of 3,000 vph for a two-lane on-ramp that does not taper to one merge lane and for a two-lane off-ramp with two auxiliary lanes.

Performance Standard

Level of Service E (peak hour V/C less than or equal to 1.00).

Mitigation Requirement

For V/C greater than the acceptable level of service, mitigation of the project contribution is required to bring ramp back to acceptable level of service or to no-project conditions if project contribution is .02 or greater for all ramps in the study area.

III. Freeway/Tollway Mainline Segments

V/C Calculation Methodology

Level of service to be based on peak hour V/C ratios calculated using the following capacities:

2,000 vehicles per hour per lane (vphpl) for mixed-flow (general purpose) lanes.

1,600 vphpl for a one-lane buffer-separated HOV facility.

1,750 vphpl for a two-lane buffer-separated HOV facility.

Performance Standard

Level of Service E (peak hour V/C less than or equal to 1.00).

(continued)

Table 5 (cont)

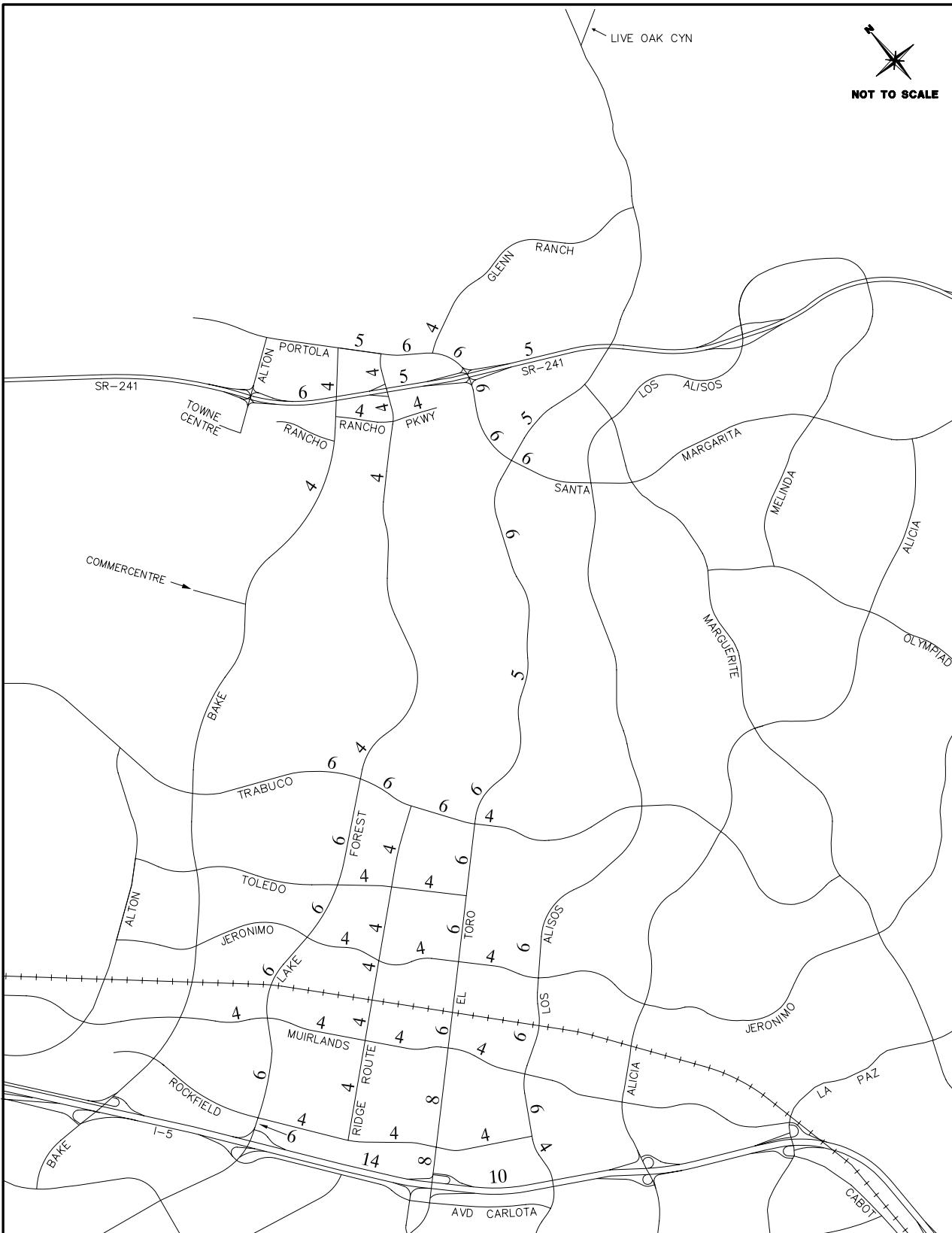
PERFORMANCE CRITERIA FOR LOCATIONS ANALYZED WITHIN THE STUDY AREA

III. Freeway/Tollway Mainline Segments

Mitigation Requirement

For V/C greater than the acceptable level of service, mitigation of the project contribution is required to bring freeway/tollway mainline location back to acceptable level of service or to no-project conditions if project contribution is greater than .03 (the impact threshold specified in the CMP).

Abbreviations: CMP – Orange County Congestion Management Program



Legend

XX Midblock lanes

Figure 6

EXISTING CIRCULATION SYSTEM
WITHIN STUDY AREA

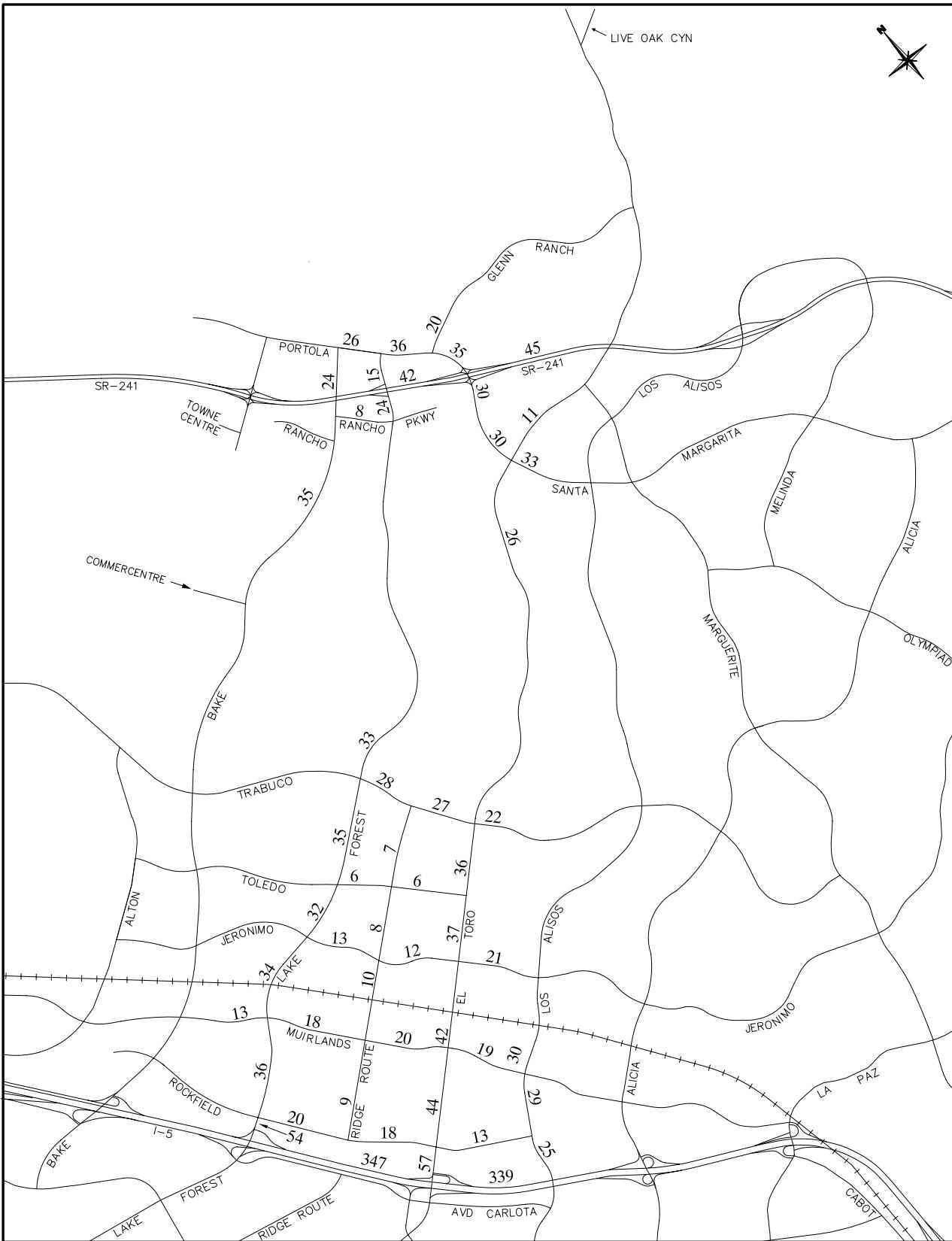
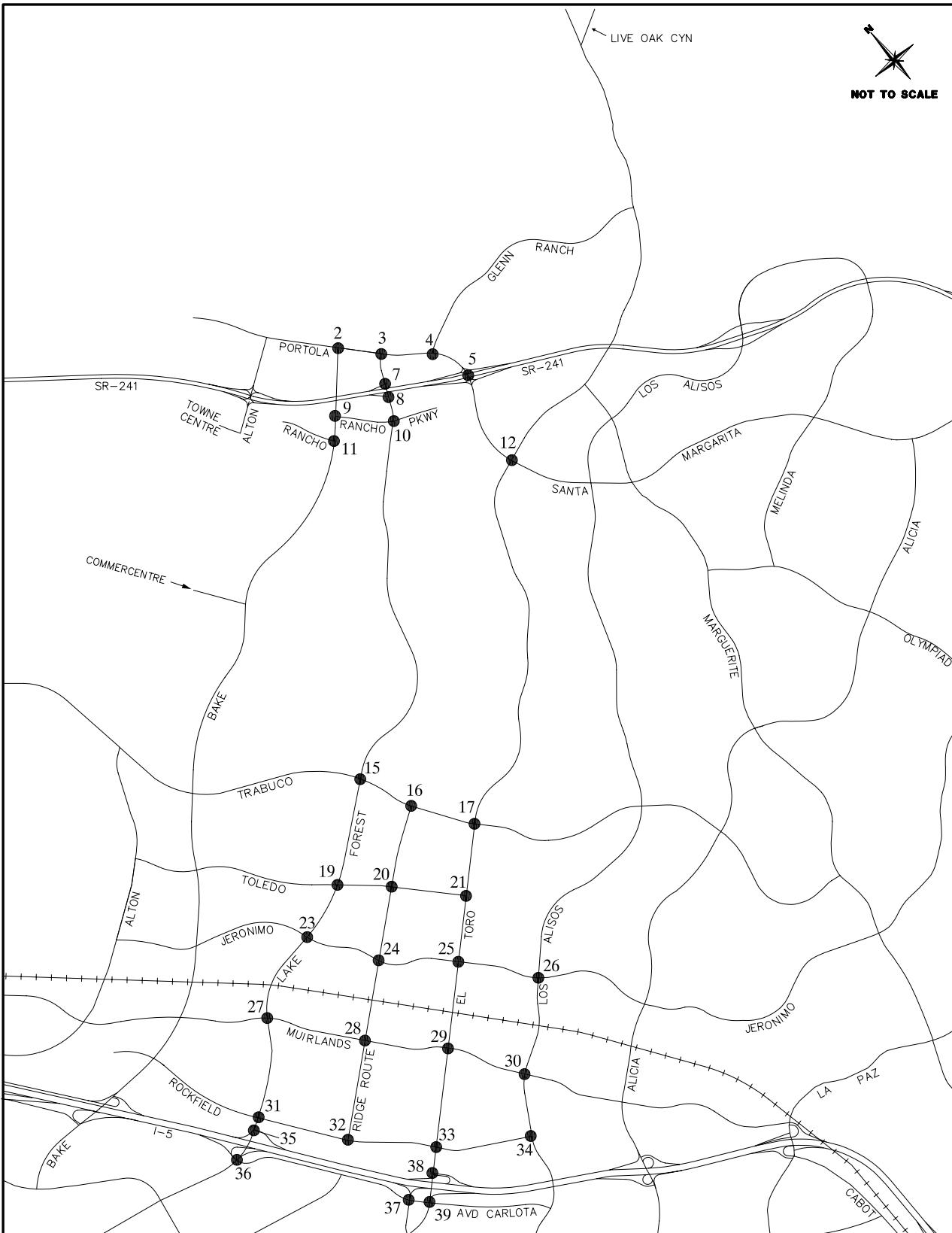


Figure 7
EXISTING ADT VOLUMES (000s)



Legend

●^X Intersection location

Figure 8

EXISTING INTERSECTION LOCATION MAP

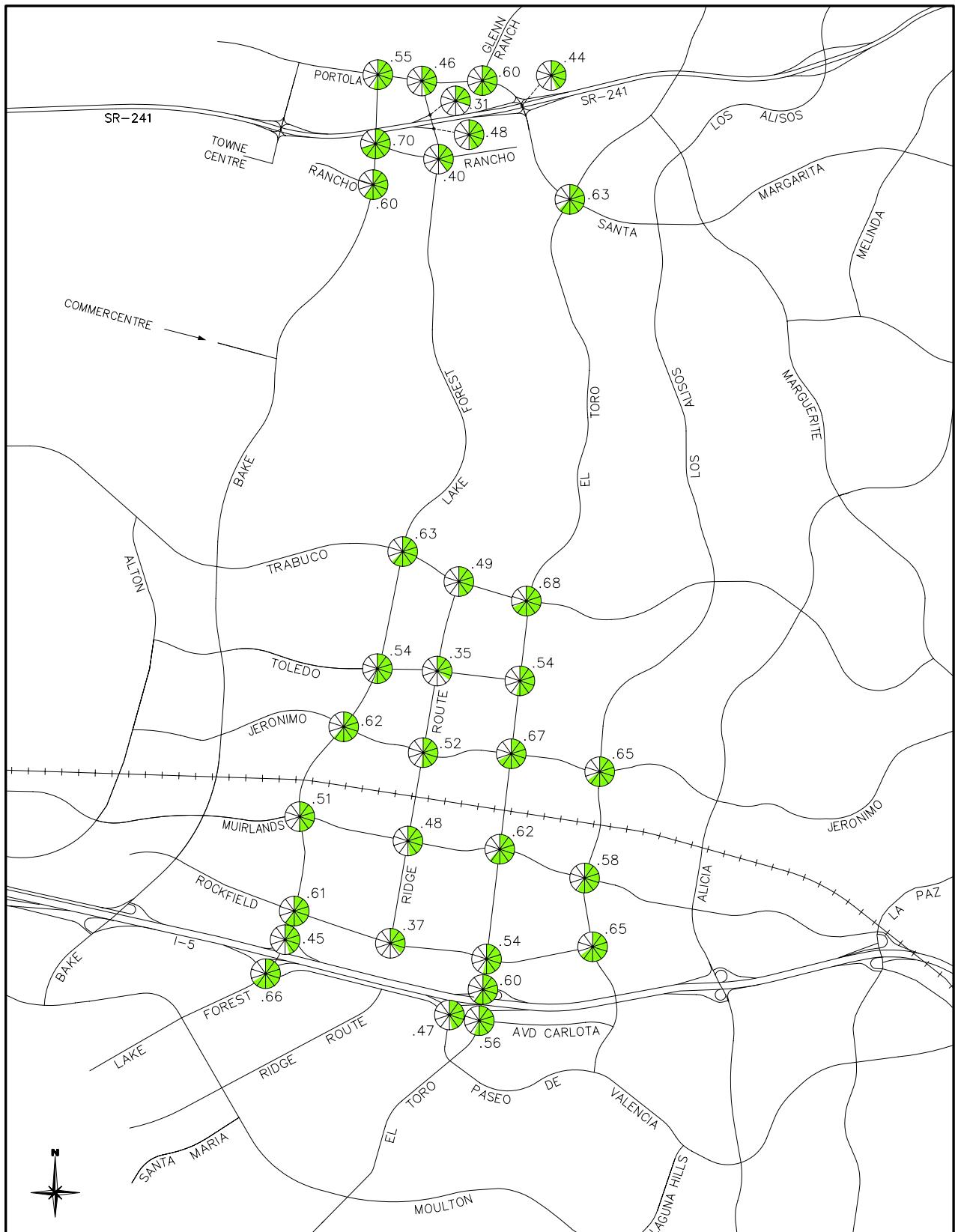
Table 6

EXISTING (2008) INTERSECTION LOS SUMMARY

Loc. # North-South (NS) Road at East-West (EW) Road	AM Peak Hour		PM Peak Hour	
	ICU	LOS	ICU	LOS
2. Bake & Portola	.55	A	.61	B
3. Lake Forest & Portola	.46	A	.65	B
4. Glenn Ranch & Portola	.60	A	.55	A
5. Portola & SR-241 Ramps	.44	A	.63	B
7. Lake Forest & SR-241 NB Ramp	.31	A	.38	A
8. Lake Forest & SR-241 SB Ramp	.48	A	.45	A
9. Bake & Rancho N	.70	B	.66	B
10. Lake Forest & Rancho	.40	A	.47	A
11. Bake & Rancho S	.60	A	.74	C
12. El Toro & Portola/Santa Margarita	.63	B	.66	B
15. Lake Forest & Trabuco	.63	B	.65	B
16. Ridge Route & Trabuco	.49	A	.58	A
17. El Toro & Trabuco	.68	B	.62	B
19. Lake Forest & Toledo	.54	A	.49	A
20. Ridge Route & Toledo	.35	A	.28	A
21. El Toro & Toledo	.54	A	.47	A
23. Lake Forest & Jeronimo	.62	B	.64	B
24. Ridge Route & Jeronimo	.52	A	.49	A
25. El Toro & Jeronimo	.67	B	.82	D
26. Los Alisos & Jeronimo	.65	B	.79	C
27. Lake Forest & Muirlands	.51	A	.45	A
28. Ridge Route & Muirlands	.48	A	.59	A
29. El Toro & Muirlands	.62	B	.54	A
30. Los Alisos & Muirlands	.58	A	.71	C
31. Lake Forest & Rockfield	.61	B	.69	B
32. Ridge Route & Rockfield	.37	A	.49	A
33. El Toro & Rockfield	.54	A	.63	B
34. Los Alisos & Rockfield	.65	B	.61	B
35. Lake Forest & I-5 NB Ramps	.45	A	.62	B
36. Lake Forest & I-5/Carlota	.66	B	.70	B
37. Paseo De Valencia & Carlota	.47	A	.55	A
38. El Toro & Bridger/I-5 NB Ramps	.60	A	.66	B
39. El Toro & Avd Carlota	.56	A	.82	D

Abbreviations: ICU – intersection capacity utilization
LOS – level of service

NB – northbound
SB – southbound



Legend

- LOS A - C (<.80)
- LOS D (<.90)
- LOS E (<1.0)
- LOS F (>1.0)

Figure 9

EXISTING AM PEAK HOUR ICUs AND
LEVEL OF SERVICE

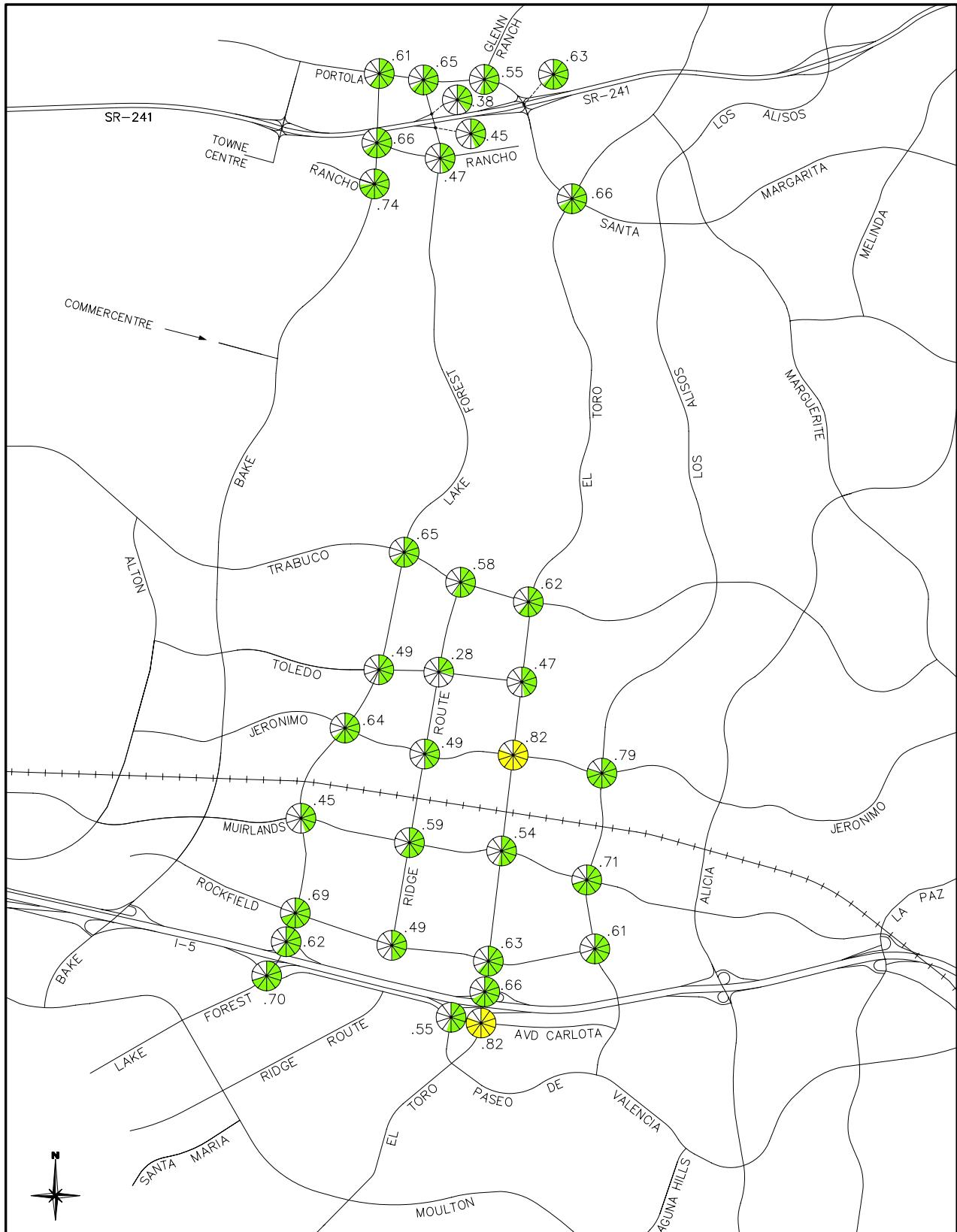


Figure 10

EXISTING PM PEAK HOUR ICUs AND
LEVEL OF SERVICE

typically determined from counts that are conducted in a 7:00 to 9:00 AM peak period and 4:00 to 6:00 PM peak period, respectively. Actual ICU worksheets can be found in Appendix C. As can be seen here, all locations are LOS “D” or better and meet the performance criteria.

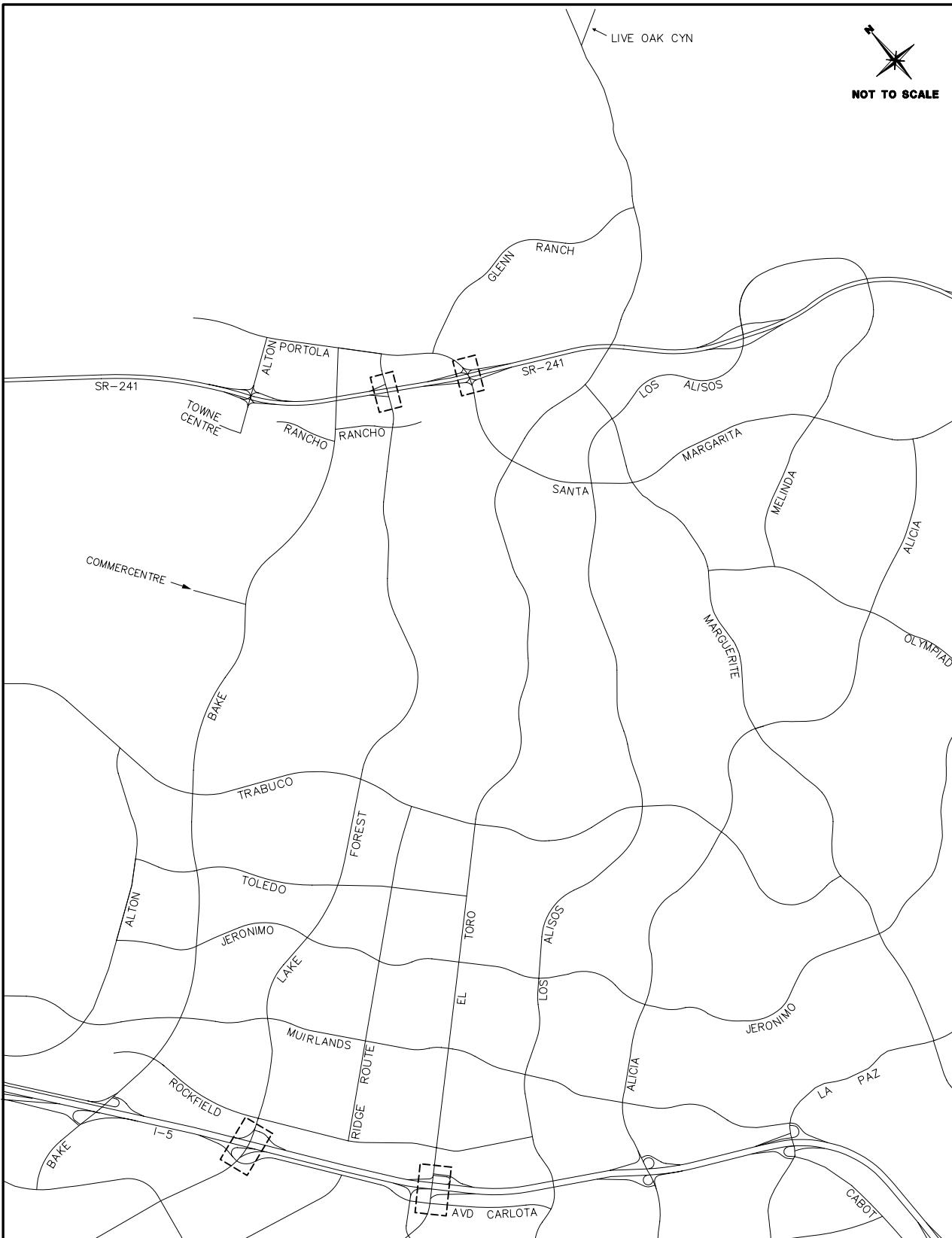
Existing Peak Hour Freeway/Tollway Ramp Levels of Service

Existing AM and PM peak hour ramp volumes were taken from intersection counts at each location in the study area where freeway/tollway ramps intersect the arterial system. The observed peak hour ramp volumes were applied together with the ramp capacities described earlier to calculate existing AM and PM peak hour ramp V/C ratios and corresponding LOSs. The freeway ramp analysis presented here, which analyzes individual ramp locations, differs from the previous peak hour intersection analysis that included ramp intersections with arterial streets. The ramp analysis involves the peak hour V/C of the ramp itself whereas the intersection analysis involves the ICU value of the ramp intersection with the arterial street. Figure 11 illustrates the interchange locations where freeway/tollway ramps were analyzed, and Table 7 summarizes existing peak hour V/C ratios for freeway/tollway ramps in the study area. The results indicate that all ramp locations in the study area currently operate better than the acceptable LOS “E” performance standard.

Existing Peak Hour Freeway/Tollway Mainline Levels of Service

To determine existing peak hour operating conditions for mainline freeway and tollway segments, peak hour traffic count data was compiled for the freeway and tollway system in the traffic analysis study area. AM and PM peak hour traffic count data was obtained from Caltrans, and that data was supplemented with AM and PM peak hour ramp volumes taken from intersection count data at locations where freeway/tollway ramps intersect the arterial system (the freeway/tollway ramp data was used to determine mainline peak hour volumes upstream and/or downstream from the locations where Caltrans counts were available).

The observed AM and PM peak hour freeway/tollway mainline volumes were applied together with the capacities previously described for mixed-flow (general purpose) lanes and high-occupancy vehicle (HOV) lanes to calculate existing peak hour V/C ratios, by direction, for freeway/tollway mainline segments in the study area. When evaluating existing freeway/tollway conditions (i.e., based on traffic count data), the V/C and LOS criteria are applicable only in situations where the observed traffic volume occurs in stable flow. When the peak hour V/C ratio on a freeway/tollway mainline segment



Legend



Freeway/tollway ramp interchange location

Figure 11

EXISTING INTERCHANGE LOCATIONS

Table 7

EXISTING FREEWAY/TOLLWAY RAMP LOS SUMMARY

Interchange	Ramp	Lanes	Peak Hour Capacity	AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS	Volume	V/C	LOS
I-5 at Lake Forest	SB Direct On	1	1,500	291	.19	A	509	.34	A
	SB Loop On	1	1,080	446	.41	A	399	.37	A
	NB On	2	1,800	906	.50	A	783	.44	A
	SB Off	2	3,000	1,730	.58	A	2,309	.77	C
	NB Off	1	1,500	984	.66	B	568	.38	A
I-5 at El Toro	SB Direct On	1	1,080	245	.23	A	507	.47	A
	SB Loop On	1	1,500	595	.40	A	834	.56	A
	NB Direct On	1	1,500	1,236	.82	D	948	.63	B
	NB Loop On	1	1,080	707	.47	A	898	.60	A
	SB Off	2	3,000	1,536	.51	A	1,263	.42	A
	NB Off	1	1,500	893	.60	A	1,198	.80	C
SR-241 at Lake Forest	NB On	2	2,250	170	.08	A	424	.19	A
	SB Off	1	1,500	533	.36	A	212	.14	A
SR-241 at Portola (East)	SB On	1	1,500	249	.17	A	747	.50	A
	NB On	2	2,250	763	.34	A	321	.14	A
	SB Off	1	1,500	251	.17	A	516	.34	A
	NB Off	2	2,250	961	.43	A	326	.14	A

nears 1.0, unstable conditions can occur which may result in a breakdown in traffic flow. This breakdown in flow causes a reduction in capacity (vehicle speeds drop below the speed at which maximum capacity is available), and hence the V/C increases, causing a further reduction in speed. The result is stop-and-go conditions. At the same time, the reduction in capacity and increase in V/C causes queue build-up and the stop-and-go conditions can extend for a considerable distance upstream of the problem freeway/tollway segment. Furthermore, this occurrence, and its severity (i.e., length of queue), can vary from day to day even when day-to-day fluctuations in traffic volumes are relatively small.

Speed and travel time measurements taken by Caltrans for the freeway/tollway system give a measure of when and where such conditions occur (i.e., for the day or days on which such measurements are taken). Specific LOS values are assigned based on the measured speeds, the LOS being derived by comparing the measured speed with a minimum desirable operating speed (typically 35 mph). The travel time studies also reveal deficient freeway/tollway segments that are not in themselves a capacity problem, but which are adversely affected by queue build-up from a deficient segment downstream. Hence, LOS values as determined from speed measurements may not equate to the V/C because a queue can extend back from a deficient segment to a segment with a relatively low V/C.

For these reasons, the V/C LOS is not always a true indication of the actual operating LOS on a freeway/tollway segment, particularly when a high V/C ratio on a given segment adversely affects upstream segments because of queue build-up. The upstream segment may have a relatively low V/C and thereby imply satisfactory operating conditions, but stop-and-go conditions extending back to this segment would cause it to actually be operating under congested conditions.

Table 8 summarizes existing AM and PM peak hour V/C ratios for freeway/tollway mainline segments in the study area. The table shows the LOSs derived from the V/C ratios together with operating LOSs determined from Caltrans field measurements as summarized in the 2007 Orange County Congestion Management Program (Orange County Transportation Authority, 2007 Edition). The existing peak hour freeway/tollway mainline segment V/C and/or speed/travel time survey LOS analysis results indicate that I-5 in the study area currently operates at LOS “F” (i.e., worse than the LOS “E” performance standard).

Note that future traffic volumes presented in this report represent “demand” and no attempt is made to estimate operating conditions such as discussed here (i.e., only the V/C LOS based on the future demand traffic volume is reported).

Table 8

EXISTING FREEWAY/TOLLWAY MAINLINE PEAK HOUR LOS SUMMARY

Location	Direction	Lanes	Peak Hour Capacity	AM Peak Hour				PM Peak Hour			
				Volume	V/C	V/C LOS	Caltrans LOS (a)	Volume	V/C	V/C LOS	Caltrans LOS (a)
I-5 n/o Lake Forest	Northbound	8+2H	19,500	14,570	.75	D	F ⁰	11,200	.57	C	E
	Southbound	8+2H	19,500	10,420	.53	C	E	14,780	.76	D	F ²
I-5 n/o El Toro	Northbound	6+2H	15,500	14,648	.95	E	F ⁰	10,985	.71	C	E
	Southbound	6+2H	15,500	9,427	.61	C	E	13,379	.86	D	F ³
I-5 n/o Alicia	Northbound	4+1H	9,600	13,598	1.42	F	F ³	10,337	1.08	F	E
	Southbound	4+1H	9,600	8,731	.91	E	E	13,457	1.40	F	F ³
SR-241 n/o Lake Forest	Northbound	3	6,000	4,560	.76	D	D	1,630	.27	A	B
	Southbound	3	6,000	1,290	.22	A	B	3,730	.62	C	D
SR-241 n/o Portola East	Northbound	3	6,000	4,390	.73	D	D	1,206	.20	A	B
	Southbound	2	4,000	757	.19	A	B	3,518	.88	D	D
SR-241 n/o Los Alisos	Northbound	3	6,000	4,588	.76	D	D	1,211	.20	A	B
	Southbound	2	4,000	755	.19	A	B	3,749	.94	E	D

Abbreviations: H – high-occupancy vehicle lane

LOS – level of service

V/C – volume/capacity ratio

(a) Caltrans LOS values are from speed and travel time surveys carried out by Caltrans as summarized in the 2007 Orange County Congestion Management Program. The measured speeds in each segment reflect queue build-up from a downstream deficient segment and/or other prevailing conditions at the time the surveys were conducted. The superscript values for LOS "F" (i.e., 0, 1, 2, 3) represent different lengths of time during which congested conditions occur in the peak period.

PLANNED CIRCULATION SYSTEM

Figures 12 and 13 show the midblock travel lanes on individual arterial road and freeway/tollway mainline segments of the study area circulation system for short-term conditions year 2011 and year 2015 cumulative. The short-term circulation system for year 2011 used in this analysis assumes the current existing circulation system for the study area. In the cumulative analysis year 2015, Alton Parkway is assumed to be connected between Towne Centre Drive and Irvine Boulevard. For worst-case analysis purposes, the proposed project is assumed to be built out under with-project year 2011 and year 2015 cumulative conditions along with the extension of Rancho Parkway to Portola Parkway from its current terminus just east of Lake Forest Drive as a project feature. The new intersection formed at Portola Parkway and Rancho Parkway will be analyzed (lane geometrics assumed are presented in Appendix C).

TRAFFIC IMPACTS WITHIN STUDY AREA

In this section, future levels of service on the study area circulation system are summarized for year 2011 and year 2015 cumulative conditions assuming buildout of the proposed project. Traffic volumes and performance evaluation results for conditions with and without the proposed project land uses. Project impacts are identified by applying the performance criteria outlined earlier in this report. Should the project require mitigation measures for year 2011 and/or year 2015 cumulative, reference will be first made of any LFTM improvements because the proposed project is part of the OSA Program.

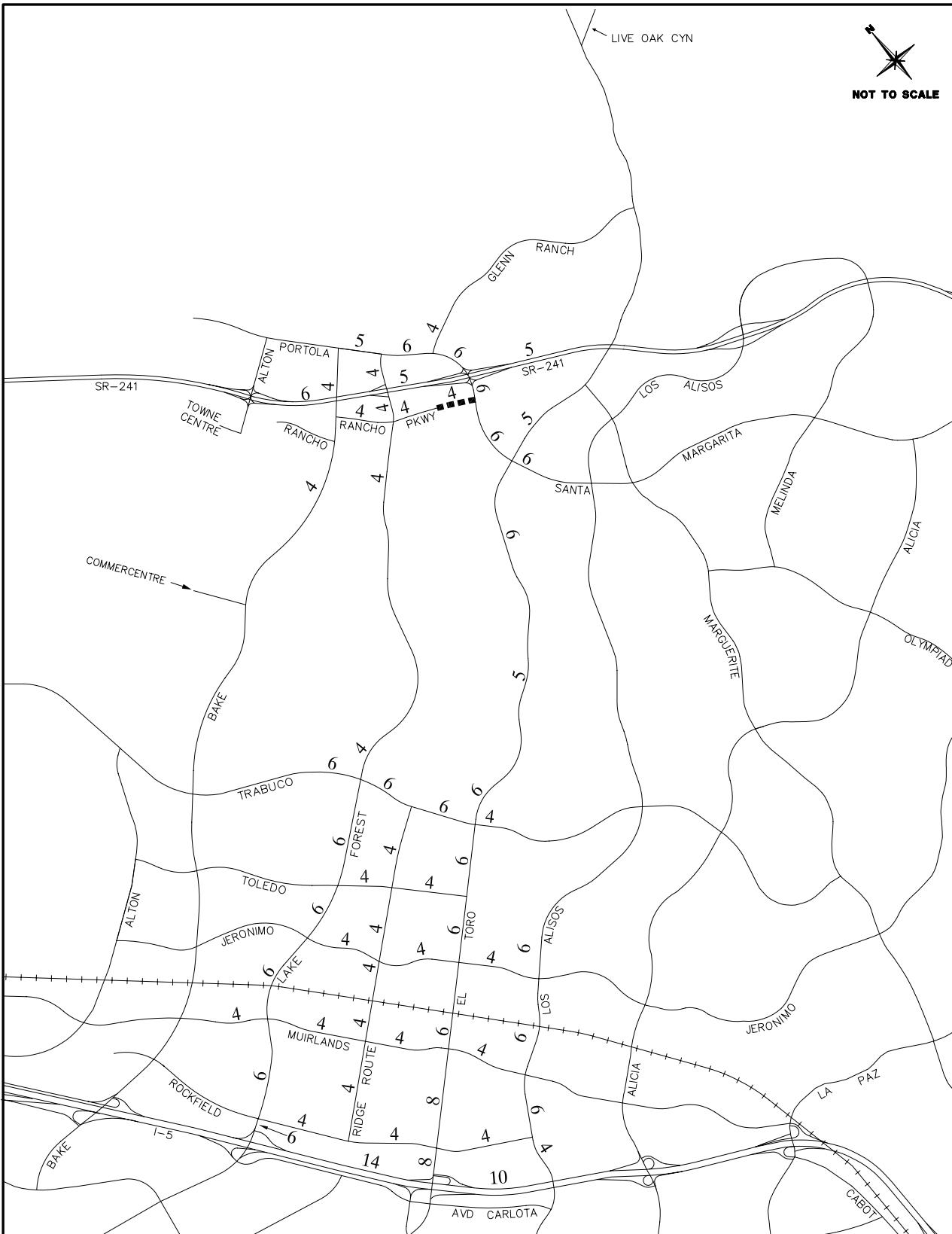
Short-Term (Year 2011) Average Daily Traffic Volumes

Short-term (year 2011) ADT forecasts are illustrated in Figures 14 and 15 for no-project and with-project conditions, respectively. The roadway network used here is the current existing circulation system with Rancho Parkway extended to Portola Parkway under with-project conditions.

Short-Term (Year 2011) Peak Hour Intersection Levels of Service

Figure 16 illustrates the intersection locations that were analyzed based on short-term (year 2011) traffic conditions, and Table 9 summarizes the AM and PM peak hour ICU values and the corresponding LOS for with and without project conditions. The ICUs are also illustrated in Figures 17 through 20. Actual turn volumes and ICU calculation worksheets are provided in Appendix C.

(Text continued on page 41)

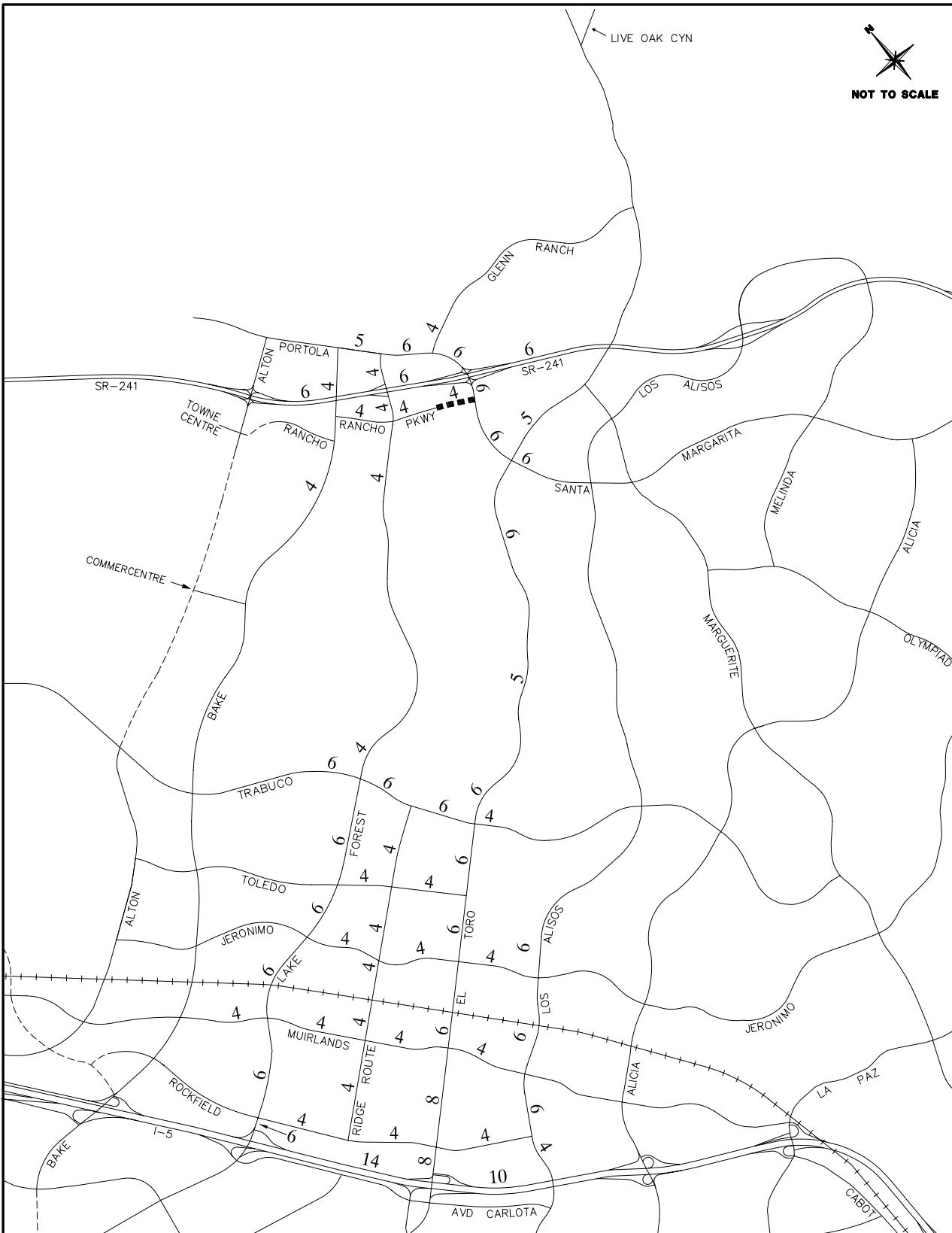


Legend

- Roadway in with-project only
- XX Midblock lanes

Figure 12

SHORT-TERM (YEAR 2011) CIRCULATION SYSTEM
WITHIN STUDY AREA



Legend

- Future roadway in by 2015
- Future roadway in with-project only
- XX Midblock lanes

Figure 13
SHORT-TERM (YEAR 2015 CUMULATIVE)
CIRCULATION SYSTEM WITHIN STUDY AREA

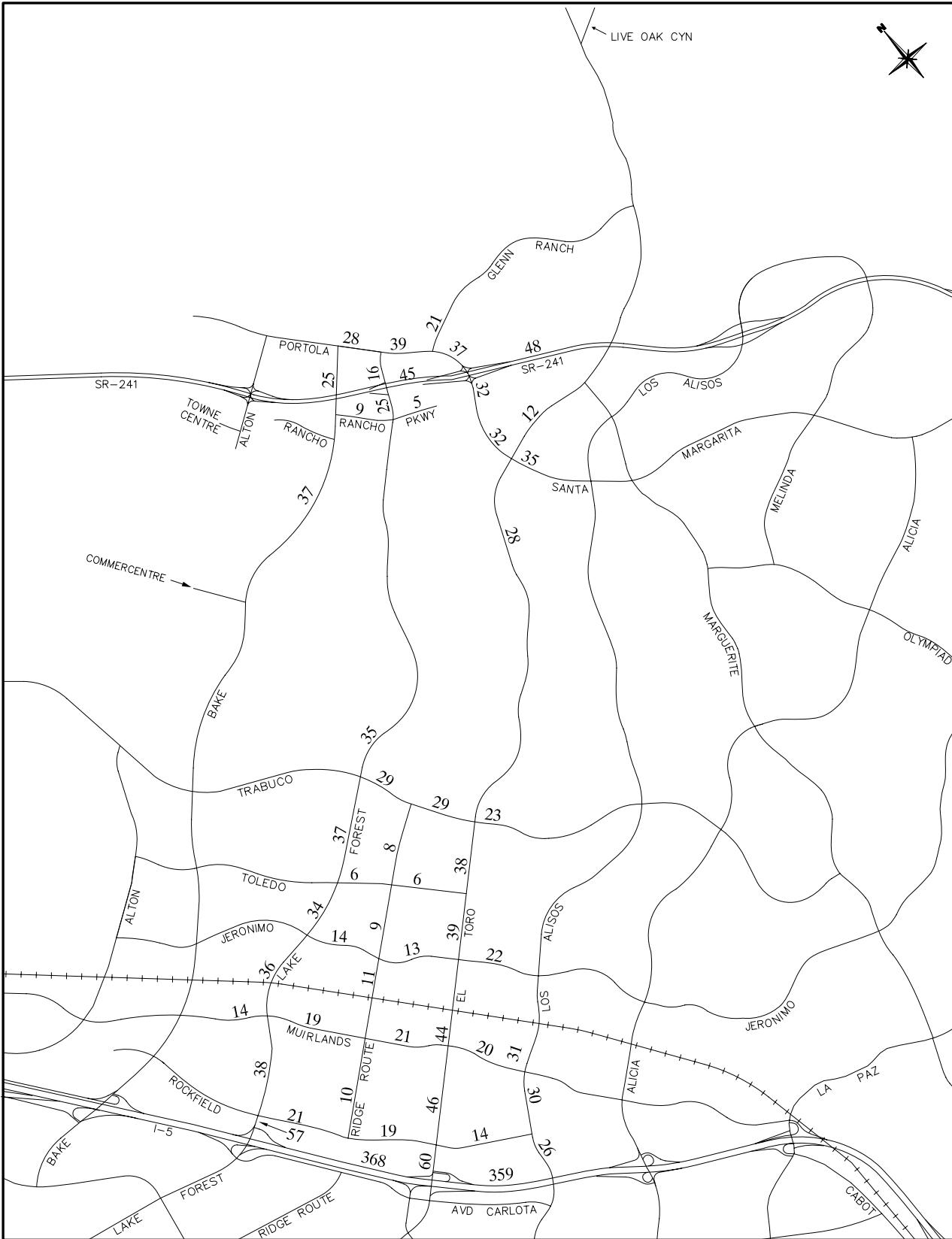
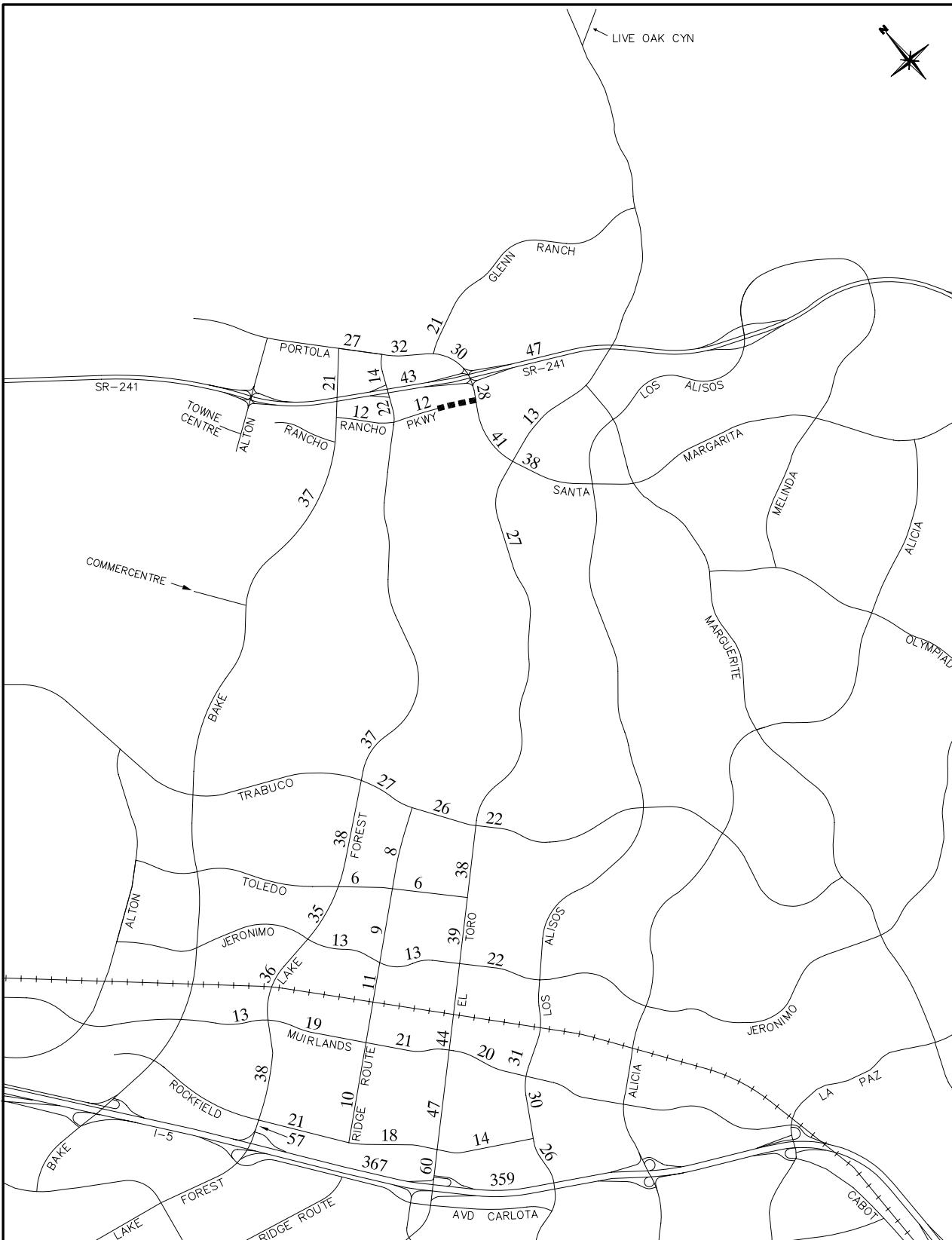


Figure 14
SHORT-TERM (YEAR 2011) ADT VOLUMES (000s)
- NO-PROJECT



Legend

- Existing Roadway
- Future roadway in with-project only

Figure 15

SHORT-TERM (YEAR 2011) ADT VOLUMES (000s)
- WITH-PROJECT



Legend

- X Intersection location
- Future roadway in with-project only

Figure 16

SHORT-TERM (YEAR 2011)
INTERSECTION LOCATION MAP

Intersection	No-Project								With-Project		Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour					
	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM		
2. Bake & Portola	.59	A	.65	B	.56	A	.63	B	-.03	-.02		
3. Lake Forest & Portola	.49	A	.68	B	.44	A	.55	A	-.05	-.13		
4. Glenn Ranch & Portola	.64	B	.57	A	.55	A	.54	A	-.09	-.03		
5. Portola & SR-241 Ramps	.46	A	.65	B	.43	A	.56	A	-.03	-.09		
7. Lake Forest & SR-241 NB	.34	A	.40	A	.29	A	.35	A	-.05	-.05		
8. Lake Forest & SR-241 SB	.51	A	.48	A	.42	A	.41	A	-.09	-.07		
9. Bake & Rancho North	.74	C	.70	B	.80	C	.70	B	.06	.00		
10. Lake Forest & Rancho	.42	A	.49	A	.62	B	.77	C	.20	.28		
11. Bake & Rancho South	.63	B	.78	C	.64	B	.80	C	.01	.02		
12. El Toro & Portola/Santa Margarita	.66	B	.67	B	.78	C	.76	C	.12	.09		
15. Lake Forest & Trabuco	.68	B	.68	B	.70	B	.69	B	.02	.01		
16. Ridge Route & Trabuco	.51	A	.61	B	.50	A	.60	A	-.01	-.01		
17. El Toro & Trabuco	.72	C	.70	B	.69	B	.69	B	-.03	-.01		
19. Lake Forest & Toledo	.55	A	.52	A	.55	A	.50	A	.00	-.02		
20. Ridge Route & Toledo	.37	A	.31	A	.36	A	.30	A	-.01	-.01		
21. El Toro & Toledo	.57	A	.49	A	.59	A	.49	A	.02	.00		
23. Lake Forest & Jeronimo	.65	B	.68	B	.65	B	.66	B	.00	-.02		
24. Ridge Route & Jeronimo	.55	A	.51	A	.54	A	.50	A	-.01	-.01		
25. El Toro & Jeronimo	.70	B	.86	D	.69	B	.84	D	-.01	-.02		
26. Los Alisos & Jeronimo	.68	B	.84	D	.68	B	.82	D	.00	-.02		
27. Lake Forest & Muirlands	.55	A	.48	A	.55	A	.47	A	.00	-.01		
28. Ridge Route & Muirlands	.50	A	.61	B	.50	A	.61	B	.00	.00		
29. El Toro & Muirlands	.65	B	.56	A	.64	B	.55	A	-.01	-.01		
30. Los Alisos & Muirlands	.61	B	.75	C	.60	A	.76	C	-.01	.01		

Table 9 (cont.)

SHORT-TERM (YEAR 2011) INTERSECTION LOS SUMMARY WITHIN STUDY AREA

Intersection	No-Project				With-Project				Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour			
	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM
31. Lake Forest & Rockfield	.64	B	.73	C	.65	B	.74	C	.01	.01
32. Ridge Route & Rockfield	.39	A	.52	A	.39	A	.50	A	.00	-.02
33. El Toro & Rockfield	.55	A	.66	B	.57	A	.68	B	.02	.02
34. Los Alisos & Rockfield	.68	B	.66	B	.68	B	.65	B	.00	-.01
35. Lake Forest & I-5 NB	.47	A	.66	B	.48	A	.65	B	.01	-.01
36. Lake Forest & I-5/Carlota	.70	B	.73	C	.67	B	.73	C	-.03	.00
37. Paseo De Valencia & Carlota	.50	A	.58	A	.50	A	.59	A	.00	.01
38. El Toro & Bridger/I-5 NB	.63	B	.70	B	.62	B	.69	B	-.01	-.01
39. El Toro & Avd Carlota	.59	A	.85	D	.60	A	.86	D	.01	.01
40. Portola & Rancho	--	--	--	--	.49	A	.68	B	.49	.68

Abbreviations: ICU – intersection capacity utilization LOS – level of service NB – northbound SB – southbound

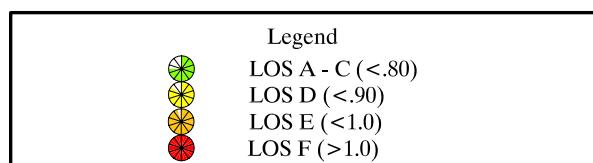
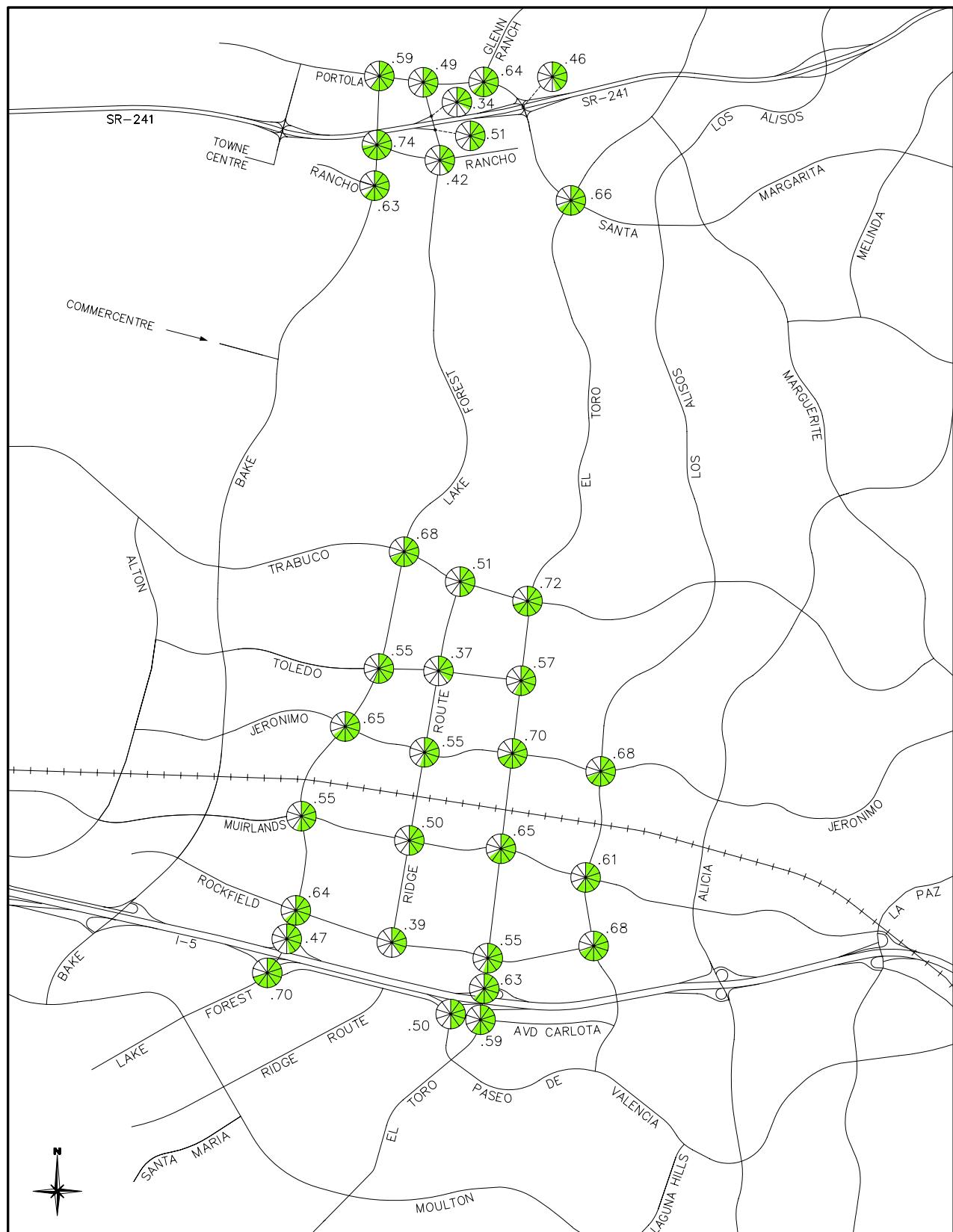
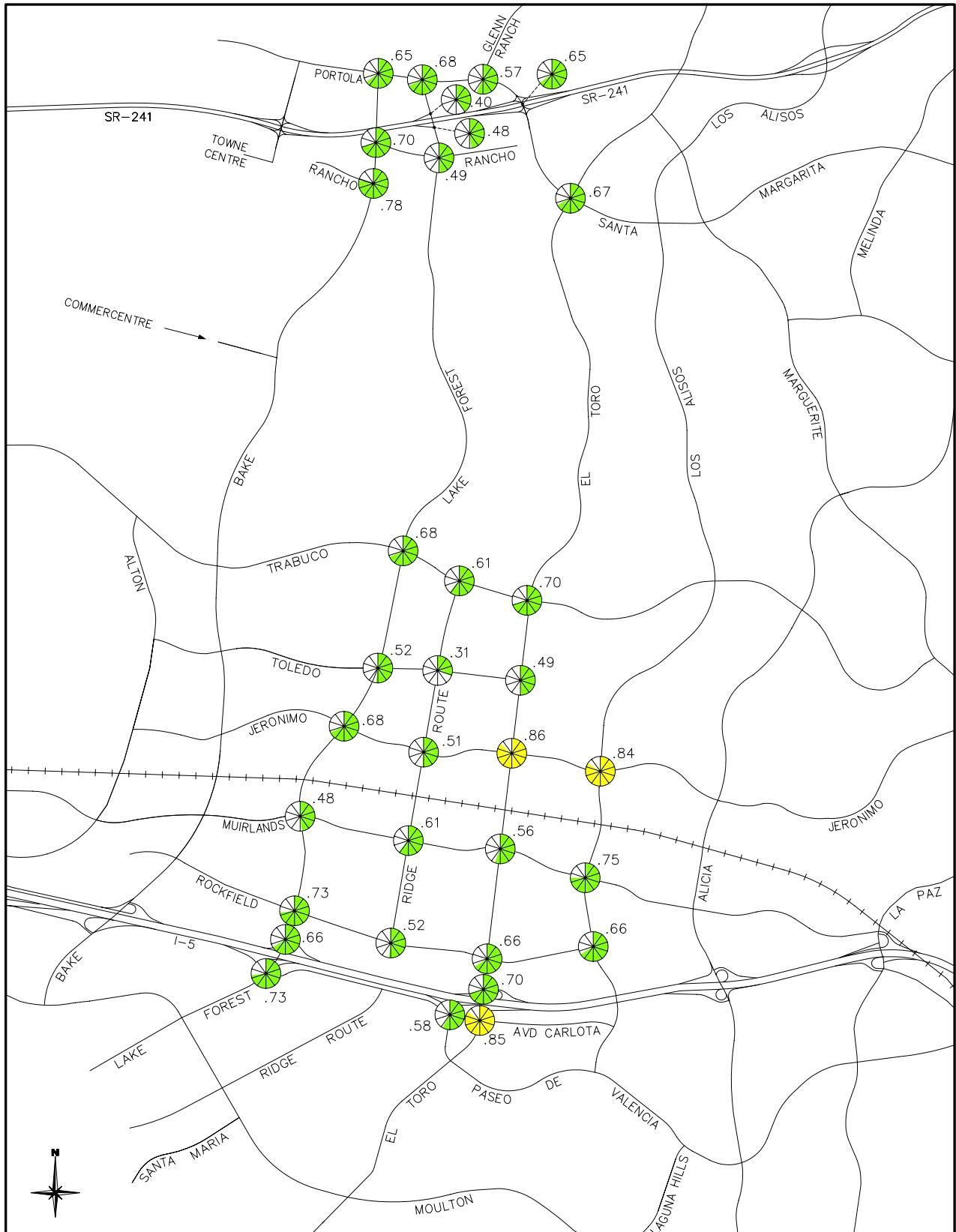


Figure 17
SHORT-TERM (YEAR 2011) AM PEAK HOUR ICUs
AND LEVEL OF SERVICE
- NO-PROJECT



Legend

 LOS A - C (<.80)
 LOS D (<.90)
 LOS E (<1.0)
 LOS F (>1.0)

Figure 18
 SHORT-TERM (YEAR 2011) PM PEAK HOUR ICUs
 AND LEVEL OF SERVICE
 - NO-PROJECT

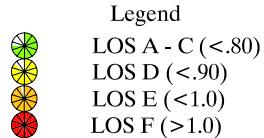
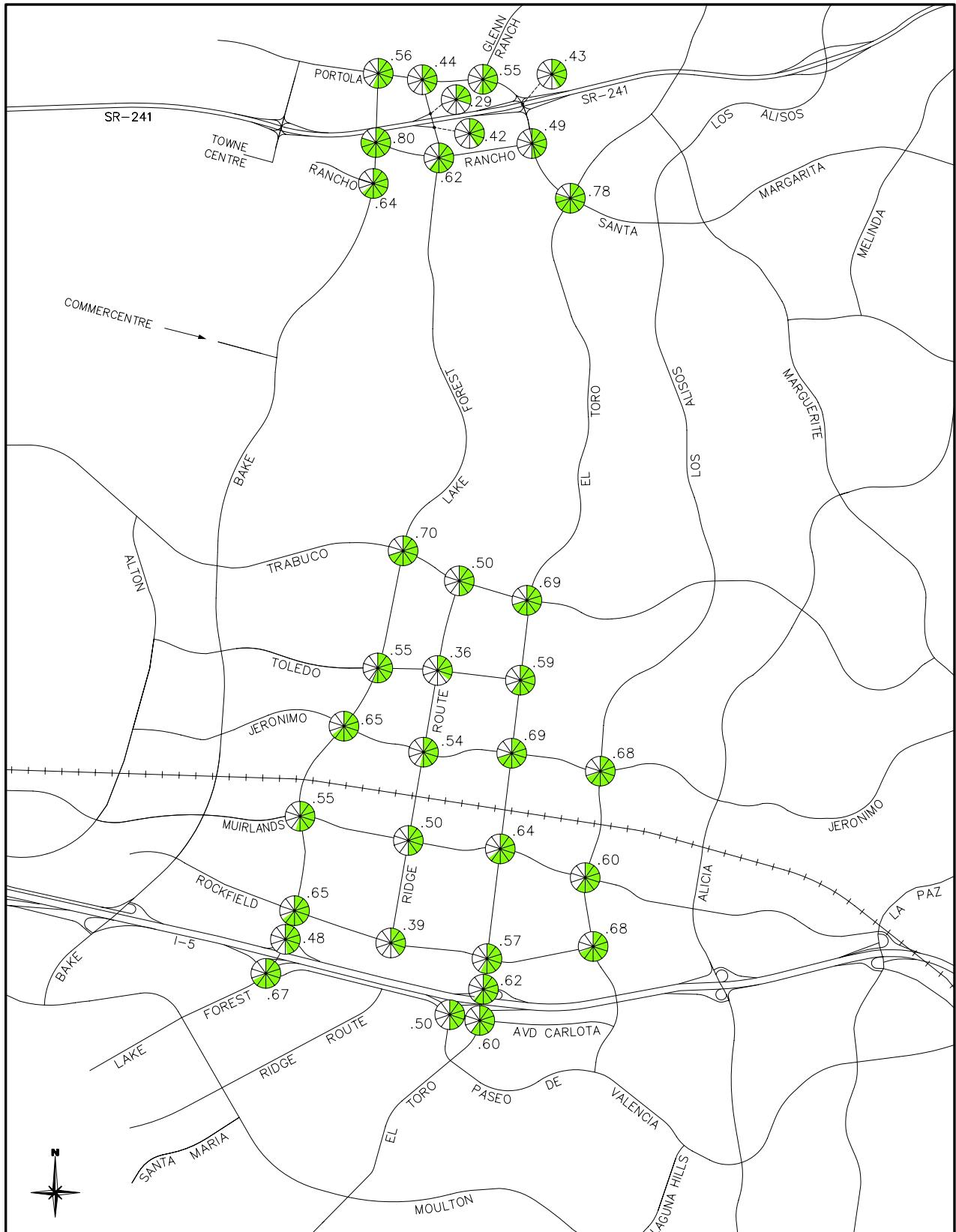


Figure 19
SHORT-TERM (YEAR 2011) AM PEAK HOUR ICUs
AND LEVEL OF SERVICE
- WITH-PROJECT

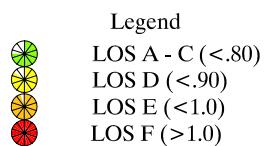
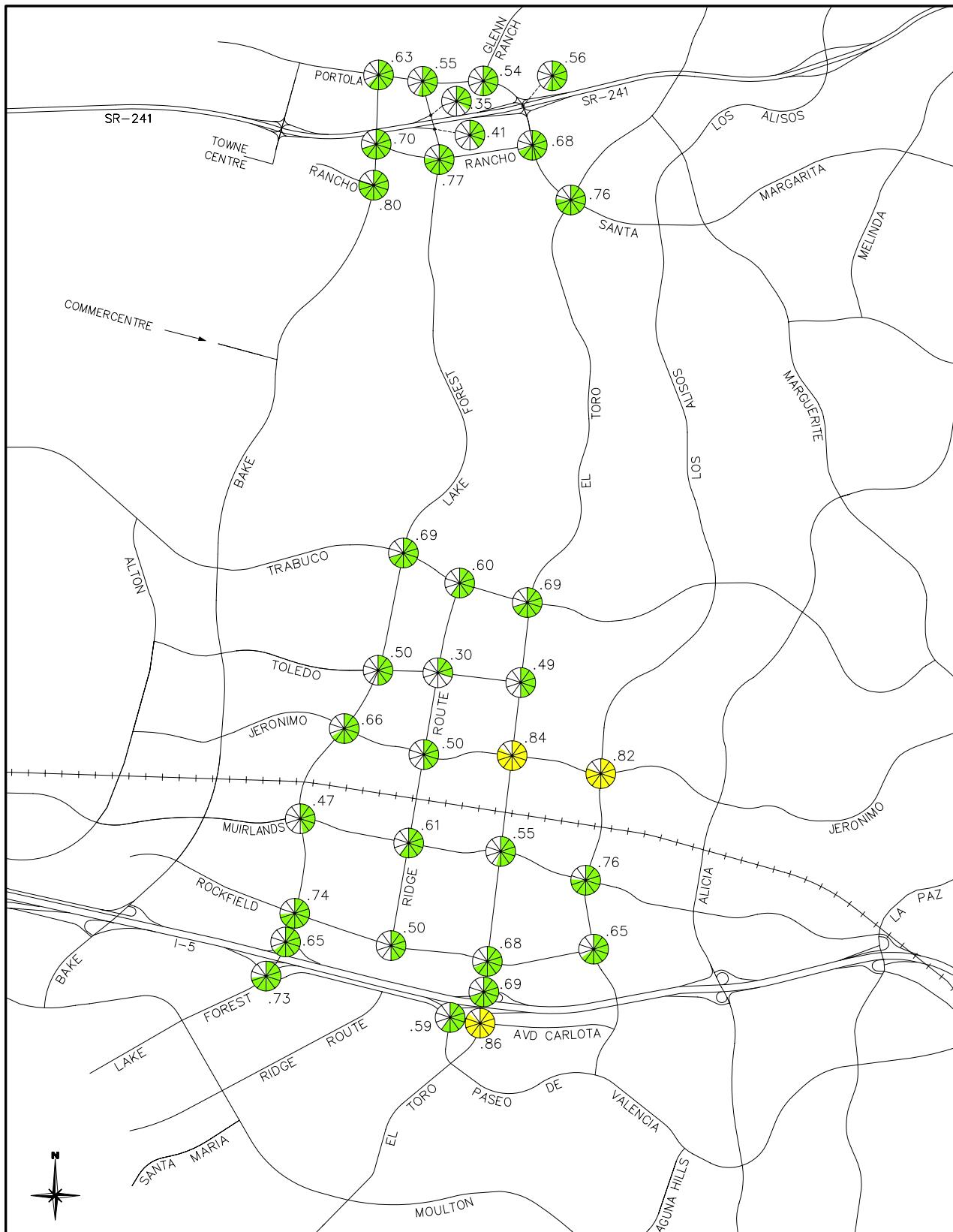


Figure 20
SHORT-TERM (YEAR 2011) PM PEAK HOUR ICUs
AND LEVEL OF SERVICE
- WITH-PROJECT

Based on the peak hour intersection performance criteria and impact thresholds established for the analysis, no intersection within the study area is significantly impacted by the proposed project land uses under short-term (year 2011) conditions.

Short-Term (Year 2011) Peak Hour Freeway/Tollway Ramp Levels of Service

Figure 21 illustrates the interchange locations where freeway ramps were analyzed based on year short-term (year 2011) conditions. Short-term (year 2011) AM and PM peak hour ramp volumes and V/C ratios for with and without project are summarized in Table 10. Based on the peak hour ramp performance criteria and impact thresholds established for the analysis, no freeway ramp is forecast to be significantly impacted by the proposed project land uses under short-term (year 2011) conditions.

Short-Term (Year 2011) Peak Hour Freeway/Tollway Mainline Levels of Service

Short-term (year 2011) with-project AM and PM freeway mainline peak hour volumes and V/C ratios for with and without project are summarized in Table 11. Based on the peak hour mainline performance criteria and impact thresholds established for the analysis, no freeway mainline segment is forecast to be significantly impacted by the proposed project land uses under year short-term (year 2011) conditions (i.e., the project does not cause LOS “F” conditions or contributes more than a .03 V/C to an already deficient LOS “F” condition). It should be noted that the LOS thresholds and significance criteria used here are from the CMP and do not necessarily represent Caltrans policy.

Short-Term (Year 2015 Cumulative) Average Daily Traffic Volumes

Short-term (cumulative analysis year 2015) ADT forecasts are illustrated in Figures 22 and 23 for no-project and with-project conditions, respectively. In the cumulative analysis year 2015, the roadway network used assumes that Alton Parkway is connected between Towne Centre Drive and Irvine Boulevard and that with-project conditions, Rancho Parkway is extended to Portola Parkway.

Short-Term (Year 2015 Cumulative) Peak Hour Intersection Levels of Service

Figure 24 illustrates the intersection locations that were analyzed based on short-term (year 2015) traffic conditions, and Table 12 summarizes the AM and PM peak hour ICU values and the corresponding



Figure 21

SHORT-TERM (YEAR 2011)
INTERCHANGE LOCATIONS

Table 10															
SHORT-TERM (YEAR 2011) FREEWAY/TOLLWAY RAMP LOS SUMMARY															
Interchange	Ramp	Lanes	Peak Hour Capacity	No-Project						With-Project					
				AM Peak Hour			PM Peak Hour			AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS									
I-5 at Lake Forest	SB Direct On	1	1,500	309	.21	A	540	.36	A	297	.20	A	550	.37	A
	SB Loop On	1	1,080	473	.44	A	423	.39	A	478	.44	A	417	.39	A
	NB On	2	1,800	960	.53	A	830	.46	A	995	.55	A	834	.46	A
	SB Off	2	3,000	1,834	.61	B	2,448	.82	D	1,808	.60	A	2,383	.79	C
	NB Off	1	1,500	1,043	.70	B	602	.40	A	1,030	.69	B	595	.40	A
I-5 at El Toro	SB Direct On	1	1,080	261	.24	A	538	.50	A	254	.24	A	531	.49	A
	SB Loop On	1	1,500	631	.42	A	883	.59	A	634	.42	A	871	.58	A
	NB Direct On	1	1,500	1,311	.87	D	1,005	.67	B	1,353	.90	D	998	.67	B
	NB Loop On	1	1,500	749	.50	A	952	.63	B	749	.50	A	969	.65	B
	SB Off	2	3,000	1,628	.54	A	1,338	.45	A	1,654	.55	A	1,327	.44	A
	NB Off	1	1,500	947	.63	B	1,270	.85	D	946	.63	B	1,277	.85	D
SR-241 at Lake Forest	NB On	2	2,250	180	.08	A	449	.20	A	208	.09	A	448	.20	A
	SB Off	1	1,500	565	.38	A	224	.15	A	546	.36	A	258	.17	A
SR-241 at Portola (East)	SB On	1	1,500	264	.18	A	792	.53	A	242	.16	A	843	.56	A
	NB On	2	2,250	809	.36	A	341	.15	A	771	.34	A	356	.16	A
	SB Off	1	1,500	266	.18	A	547	.36	A	278	.19	A	513	.34	A
	NB Off	2	2,250	1,019	.45	A	345	.15	A	1,086	.48	A	341	.15	A

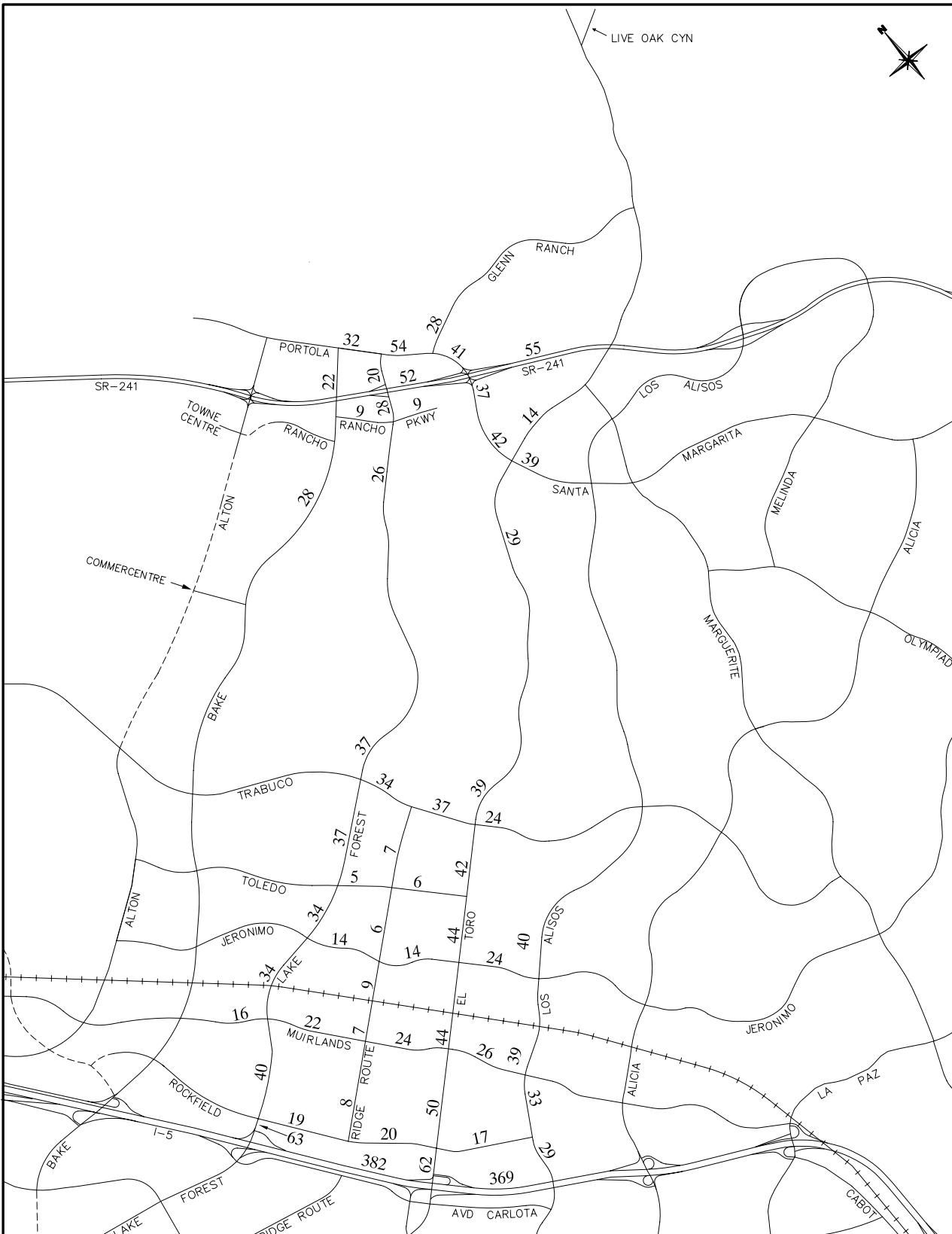
Abbreviations: LOS – level of service
 NB – northbound
 SB – southbound
 V/C – volume/capacity ratio

Table 11

SHORT-TERM (YEAR 2011) FREEWAY/TOLLWAY MAINLINE LOS SUMMARY

Location	Direction	Lanes	Peak Hour Capacity	No-Project						With-Project					
				AM Peak Hour			PM Peak Hour			AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS									
I-5 n/o Lake Forest	Northbound	8+2H	19,500	15,444	.79	D	11,872	.61	C	15,499	.79	D	11,877	.61	C
	Southbound	8+2H	19,500	11,045	.57	C	15,667	.80	D	11,045	.57	C	15,667	.80	D
I-5 n/o El Toro	Northbound	6+2H	15,500	15,527	1.00	E	11,644	.75	D	15,536	1.00	E	11,644	.75	D
	Southbound	6+2H	15,500	9,993	.64	C	14,182	.91	E	9,993	.64	C	14,188	.92	E
I-5 n/o Alicia	Northbound	4+1H	9,600	14,414	1.50	F	10,957	1.14	F	14,414	1.50	F	10,957	1.14	F
	Southbound	4+1H	9,600	9,255	.96	E	14,264	1.49	F	9,255	.96	E	14,264	1.49	F
SR-241 n/o Lake Forest	Northbound	3	6,000	4,834	.81	D	1,728	.29	A	4,834	.81	D	1,728	.29	A
	Southbound	3	6,000	1,367	.23	A	3,954	.66	C	1,367	.23	A	3,954	.66	C
SR-241 n/o Portola East	Northbound	3	6,000	4,653	.78	D	1,278	.21	A	4,653	.78	D	1,278	.21	A
	Southbound	2	4,000	802	.20	A	3,729	.93	C	818	.21	A	3,729	.93	C
SR-241 n/o Los Alisos	Northbound	3	6,000	4,863	.81	D	1,284	.21	A	4,863	.81	D	1,284	.21	A
	Southbound	2	4,000	800	.20	A	3,974	.99	C	800	.21	A	3,974	.99	C

Abbreviations: H – high-occupancy vehicle lane
 LOS – level of service
 V/C – volume/capacity ratio

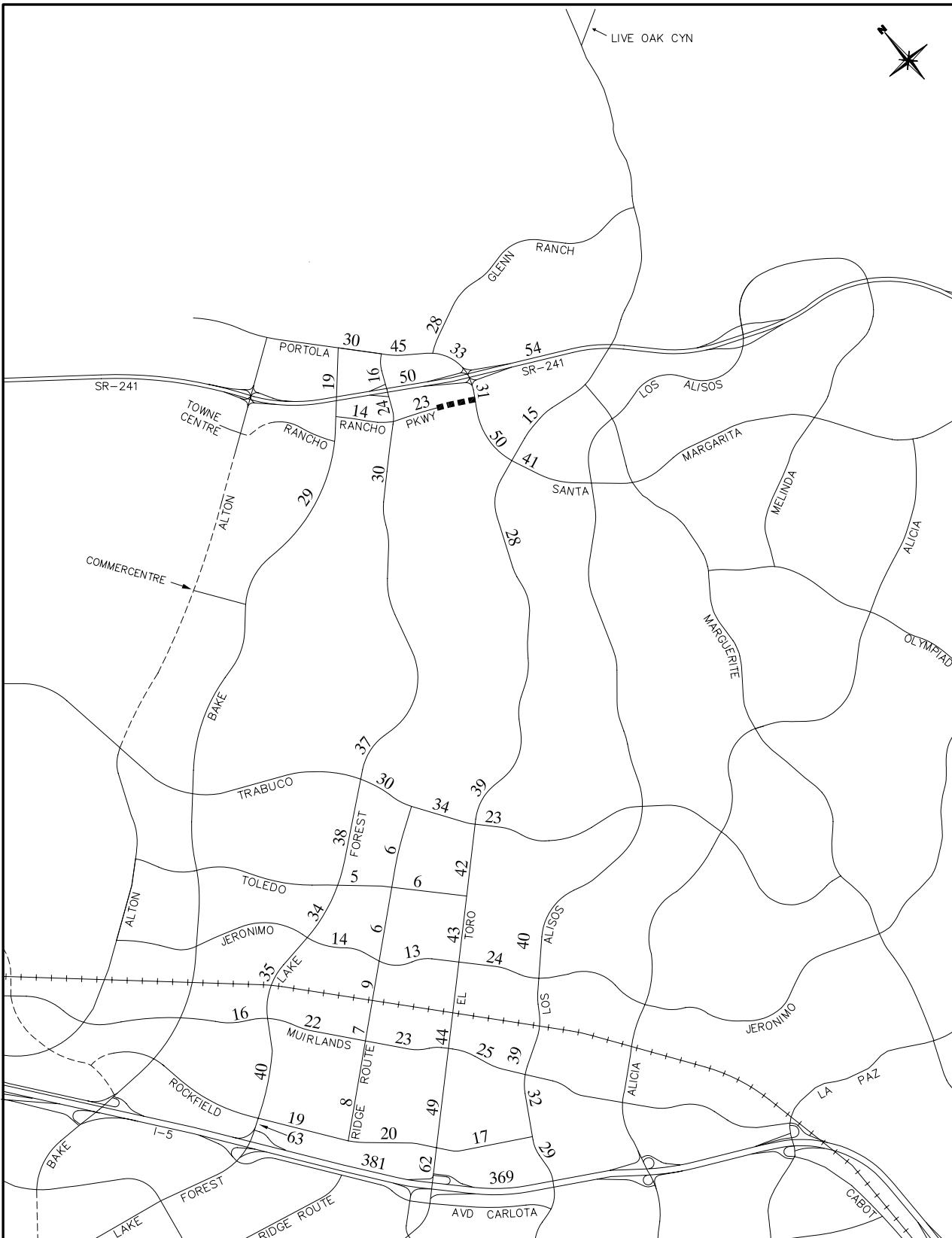


Legend

- Existing roadway
- - - Future roadway in by 2015

Figure 22

SHORT-TERM (YEAR 2015 CUMULATIVE)
ADT VOLUMES (000s)
- NO-PROJECT



Legend

- Existing roadway
- - - Future roadway in by 2015
- Future roadway in with-project only

Figure 23

SHORT-TERM (YEAR 2015 CUMULATIVE)
ADT VOLUMES (000s)
- WITH-PROJECT

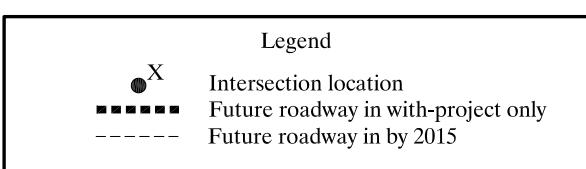
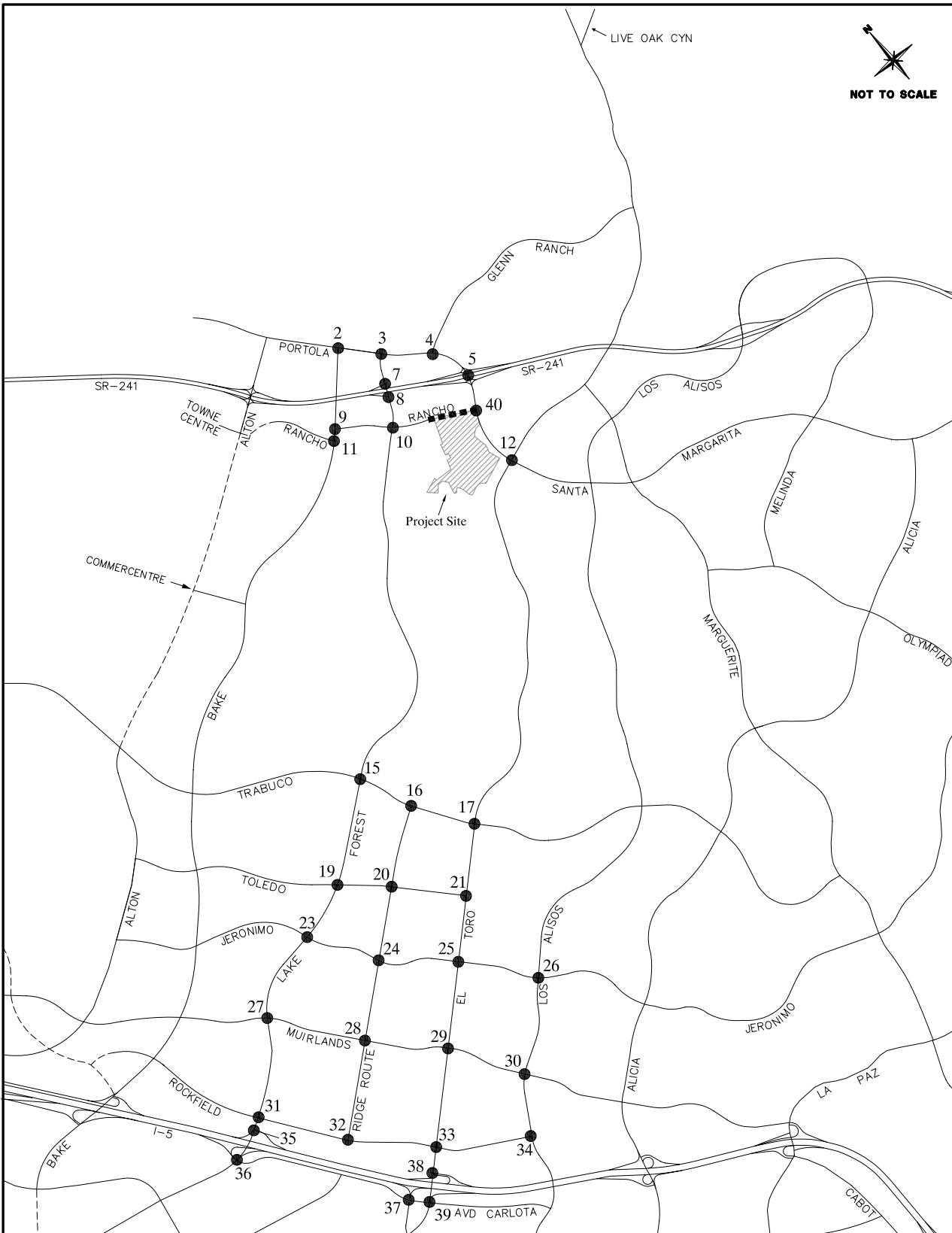


Figure 24
SHORT-TERM (YEAR 2015 CUMULATIVE)
INTERSECTION LOCATION MAP

Intersection	Table 12 SHORT-TERM (YEAR 2015 CUMULATIVE) INTERSECTION LOS SUMMARY WITHIN STUDY AREA									
	No-Project				With-Project				Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour			
Intersection	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM
2. Bake & Portola	.59	A	.82	D	.57	A	.81	D	-.02	-.01
3. Lake Forest & Portola (a)	.60	A	.99	E	.57	A	.78	C	-.03	-.21
4. Glenn Ranch & Portola	.72	C	.69	B	.63	B	.64	B	-.09	-.05
5. Portola & SR-241 Ramps	.55	A	.69	B	.48	A	.64	B	-.07	-.05
7. Lake Forest & SR-241 NB	.38	A	.44	A	.31	A	.38	A	-.07	-.06
8. Lake Forest & SR-241 SB	.50	A	.53	A	.42	A	.45	A	-.08	-.08
9. Bake & Rancho North	.58	A	.73	C	.66	B	.74	C	.08	.01
10. Lake Forest & Rancho (a) (b)	.50	A	.67	B	.61	B	.92	E	.11	.25
11. Bake & Rancho South	.62	B	.66	B	.63	B	.70	B	.01	.04
12. El Toro & Portola/Santa Margarita	.68	B	.81	D	.70	B	.89	D	.02	.08
15. Lake Forest & Trabuco	.78	C	.86	D	.81	D	.84	D	.03	-.02
16. Ridge Route & Trabuco	.52	A	.63	B	.49	A	.61	B	-.03	-.02
17. El Toro & Trabuco	.71	C	.71	C	.66	B	.68	B	-.05	-.03
19. Lake Forest & Toledo	.50	A	.47	A	.50	A	.46	A	.00	-.01
20. Ridge Route & Toledo	.32	A	.34	A	.31	A	.32	A	-.01	-.02
21. El Toro & Toledo	.56	A	.59	A	.57	A	.56	A	.01	-.03
23. Lake Forest & Jeronimo	.69	B	.74	C	.67	B	.74	C	-.02	.00
24. Ridge Route & Jeronimo	.45	A	.58	A	.43	A	.56	A	-.02	-.02
25. El Toro & Jeronimo	.78	C	.79	C	.81	D	.79	C	.03	.00
26. Los Alisos & Jeronimo	.76	C	.88	D	.77	C	.90	D	.01	.02
27. Lake Forest & Muirlands	.65	B	.85	D	.62	B	.86	D	-.03	.01
28. Ridge Route & Muirlands	.49	A	.66	B	.49	A	.66	B	.00	.00
29. El Toro & Muirlands	.64	B	.81	D	.66	B	.79	C	.02	-.02
30. Los Alisos & Muirlands (a)	.89	D	.91	E	.86	D	.91	E	-.03	.00

Table 12 (cont.)

SHORT-TERM (YEAR 2015 CUMULATIVE) INTERSECTION LOS SUMMARY WITHIN STUDY AREA

Intersection	No-Project				With-Project				Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour			
	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM
31. Lake Forest & Rockfield	.68	B	.74	C	.68	B	.76	C	.00	.02
32. Ridge Route & Rockfield	.44	A	.57	A	.44	A	.56	A	.00	-.01
33. El Toro & Rockfield	.52	A	.66	B	.52	A	.66	B	.00	.00
34. Los Alisos & Rockfield	.79	C	.81	D	.80	C	.80	C	.01	-.01
35. Lake Forest & I-5 NB	.57	A	.64	B	.57	A	.64	B	.00	.00
36. Lake Forest & I-5/Carlota	.64	B	.81	D	.64	B	.82	D	.00	.01
37. Paseo De Valencia & Carlota	.50	A	.75	C	.49	A	.74	C	-.01	-.01
38. El Toro & Bridger/I-5 NB	.64	B	.67	B	.64	B	.67	B	.00	.00
39. El Toro & Avd Carlota (a)	.61	B	.99	E	.60	A	.99	E	-.01	.00
40. Portola & Rancho	--	--	--	--	.53	A	.66	B	--	--

Abbreviations: ICU – intersection capacity utilization LOS – level of service NB – northbound SB – southbound

(a) This location is forecast to operate deficiently in the AM and/or PM peak hour under no-project and/or with-project conditions (i.e., the forecasted LOS is worse than the adopted LOS performance standard).

(b) Significantly impacted by the proposed project according to the performance criteria.

LOS for with and without project conditions. The ICUs are also illustrated in Figures 25 through 28. Actual turn volumes and ICU calculation worksheets are provided in Appendix C.

Based on the peak hour intersection performance criteria and impact thresholds established for the analysis, one intersection, Lake Forest Drive and Rancho Parkway, within the study area is significantly impacted by the proposed project land uses under short-term (year 2015 cumulative) conditions (no-project PM peak hour ICU of .67 increases to .92 for with-project).

The LFTM Program is a feature of the OSA Program which provides improvements to 18 intersections potentially impacted by future development of the OSA properties. This program includes improvements at the intersection of Lake Forest Drive and Rancho Parkway which would mitigate the project impact resulting from the Sports Park/Recreation Center project. These improvements are not considered new mitigation; rather they are included in the list of LFTM improvements and are fully funded. The analysis in this report indicates that the improvements should be implemented no later than year 2015. The improvements listed in LFTM for the intersection of Lake Forest Drive and Rancho Parkway exceed what is required to mitigate the impacts of the Sports Park/Recreation Center project. Therefore, the EIR will include a mitigation measure to ensure that the minimum improvements necessary to accommodate the proposed project (a second eastbound through lane on Rancho Parkway) will be constructed no later than year 2015.

It should be noted that three intersections are forecast to operate deficiently in both the no-project and with-project scenarios. Impacts to these intersections are not attributable to the proposed Sports Park/Recreation Center project and will be improved with implementation of the LFTM Program.

Short-Term (Year 2015 Cumulative) Peak Hour Freeway/Tollway Ramp Levels of Service

Figure 29 illustrates the interchange locations where freeway ramps were analyzed based on year short-term (year 2015 cumulative) conditions. Short-term (year 2015 cumulative) AM and PM peak hour ramp volumes and V/C ratios for with and without project are summarized in Table 13. Based on the peak hour ramp performance criteria and impact thresholds established for the analysis, no freeway ramp is forecast to be significantly impacted by the proposed project land uses under short-term (year 2015 cumulative) conditions.



Legend

- LOS A - C (<.80)
- LOS D (<.90)
- LOS E (<1.0)
- LOS F (>1.0)

Figure 25

SHORT-TERM (YEAR 2015 CUMULATIVE)
AM PEAK HOUR ICUS AND LEVEL OF SERVICE
- NO-PROJECT

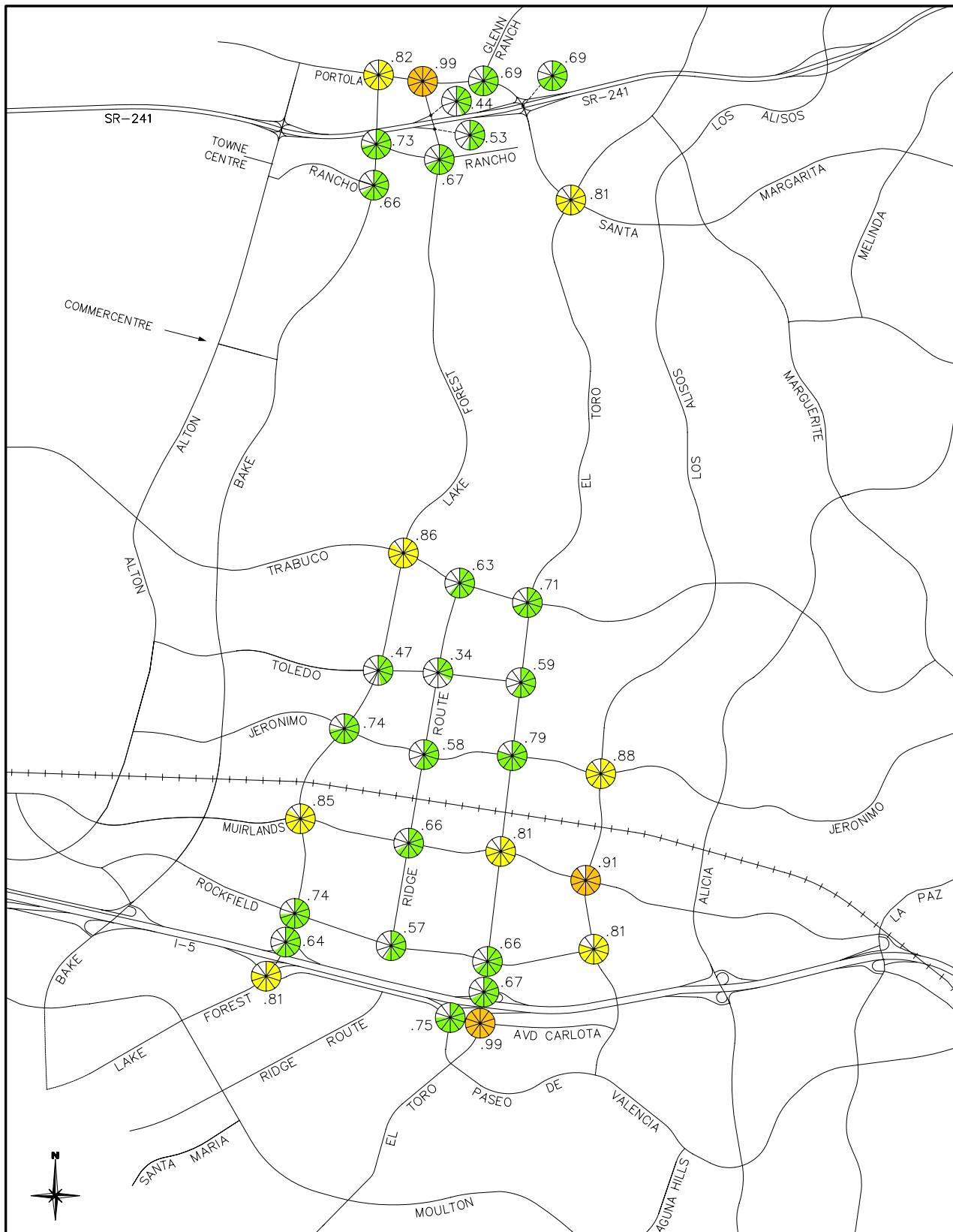


Figure 26
**SHORT-TERM (YEAR 2015 CUMULATIVE)
 PM PEAK HOUR ICUs AND LEVEL OF SERVICE
 - NO-PROJECT**



Legend

- LOS A - C (<.80)
- LOS D (<.90)
- LOS E (<1.0)
- LOS F (>1.0)

Figure 27

SHORT-TERM (YEAR 2015 CUMULATIVE)
AM PEAK HOUR ICUS AND LEVEL OF SERVICE
- WITH-PROJECT

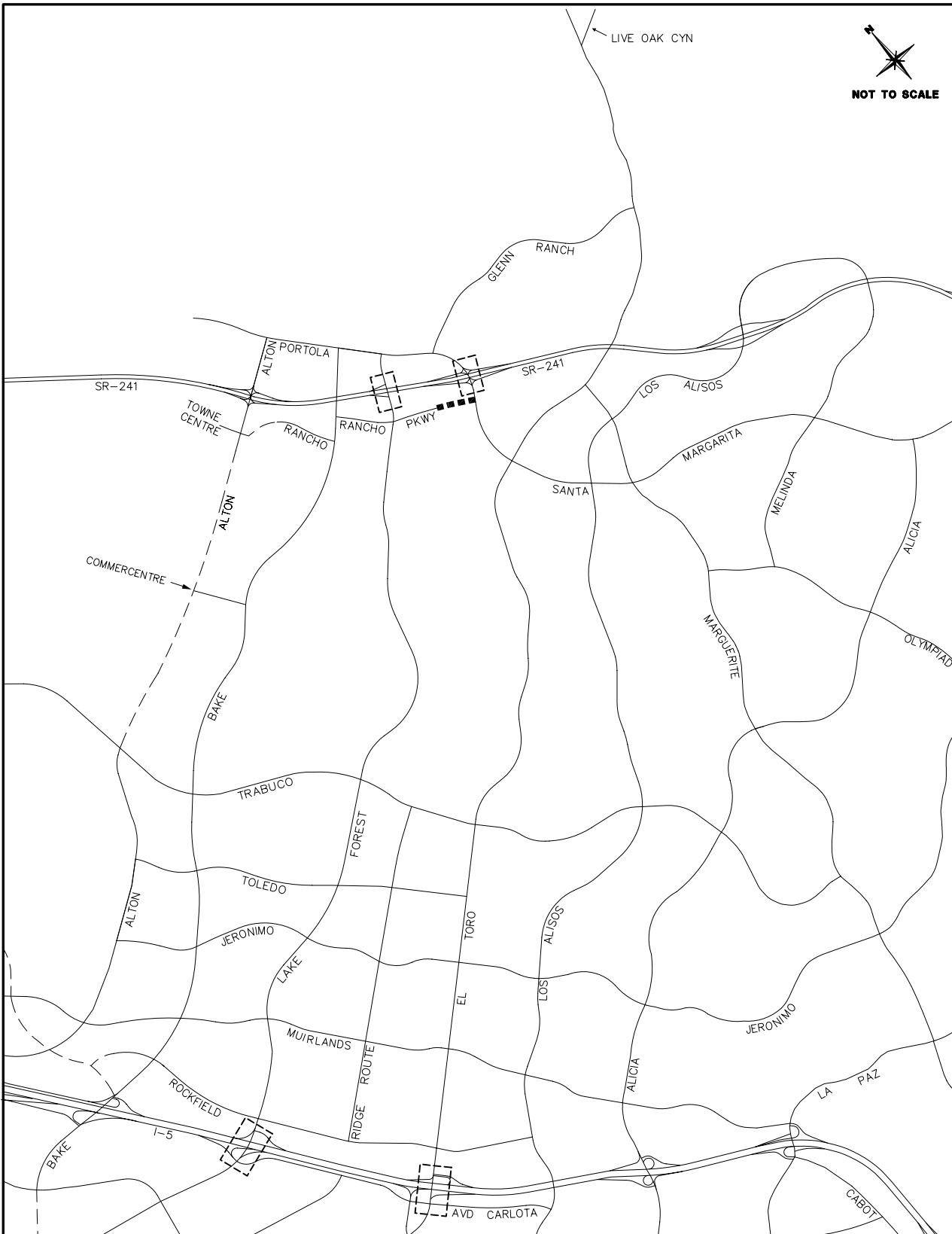


Legend

- LOS A - C (<.80)
- LOS D (<.90)
- LOS E (<1.0)
- LOS F (>1.0)

Figure 28

SHORT-TERM (YEAR 2015 CUMULATIVE)
PM PEAK HOUR ICUs AND LEVEL OF SERVICE
- WITH-PROJECT



Legend

- Existing Roadway
- - - Future roadway in by 2015
- Future roadway in with-project only
- [] Freeeway/tollway ramp interchange location

Figure 29

SHORT-TERM (YEAR 2015 CUMULATIVE)
INTERCHANGE LOCATIONS

Table 13

SHORT-TERM (YEAR 2015 CUMULATIVE) FREEWAY/TOLLWAY RAMP LOS SUMMARY

Interchange	Ramp	Lanes	Peak Hour Capacity	No-Project						With-Project					
				AM Peak Hour			PM Peak Hour			AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS	Volume	V/C	LOS	Volume	V/C	LOS	Volume	V/C	LOS
I-5 at Lake Forest	SB Direct On	1	1,500	309	.21	A	920	.61	B	297	.20	A	930	.62	B
	SB Loop On	1	1,080	550	.51	A	570	.53	A	540	.50	A	570	.53	A
	NB On	2	1,800	1,400	.78	C	1,000	.56	A	1,370	.76	C	1,000	.56	A
	SB Off	2	3,000	1,990	.66	B	3,070	1.02	F	1,990	.66	B	3,090	1.03	F
	NB Off	1	1,500	1,160	.77	C	620	.41	A	1,150	.77	C	600	.40	A
I-5 at El Toro	SB Direct On	1	1,080	261	.24	A	538	.50	A	254	.24	A	531	.49	A
	SB Loop On	1	1,500	620	.41	A	1,070	.71	C	600	.40	A	1,060	.71	C
	NB Direct On	1	1,500	1,311	.87	D	1,005	.67	B	1,353	.90	D	998	.67	B
	NB Loop On	1	1,500	910	.61	B	1,120	.75	C	910	.61	B	1,120	.75	C
	SB Off	2	3,000	1,520	.51	A	1,510	.50	A	1,540	.51	A	1,470	.49	A
	NB Off	1	1,500	1,150	.77	C	1,150	.77	C	1,140	.76	C	1,160	.77	C
SR-241 at Lake Forest	NB On	2	2,250	200	.09	A	590	.26	A	230	.10	A	600	.27	A
	SB Off	1	1,500	560	.37	A	310	.21	A	570	.38	A	320	.21	A
SR-241 at Portola (East)	SB On	1	1,500	280	.19	A	980	.65	B	250	.17	A	1,070	.71	C
	NB On	2	2,250	900	.40	A	390	.17	A	810	.36	A	370	.16	A
	SB Off	1	1,500	380	.25	A	670	.45	A	360	.24	A	600	.40	A
	NB Off	2	2,250	1,660	.74	C	390	.17	A	1,670	.74	C	400	.18	A

Abbreviations: LOS – level of service

NB – northbound

SB – southbound

V/C – volume/capacity ratio

Short-Term (Year 2015 Cumulative) Peak Hour Freeway/Tollway Mainline Levels of Service

Short-term (year 2015 cumulative) with-project AM and PM freeway mainline peak hour volumes and V/C ratios for with and without project are summarized in Table 14. Based on the peak hour mainline performance criteria and impact thresholds established for the analysis, no freeway mainline segment is forecast to be significantly impacted by the proposed project land uses under year short-term (year 2015 cumulative) conditions (i.e., the project does not cause LOS “F” conditions or contributes more than a .03 V/C to an already deficient LOS “F” condition). It should be noted that the LOS thresholds and significance criteria used here are from the CMP and do not necessarily represent Caltrans policy.

SPECIAL ISSUES

This section includes a preliminary analysis of the signalization needs for the major entrance to the sports park proposed on the planned extension of Rancho Parkway to Portola Parkway. It is considered preliminary since detailed site planning for the sports park has yet to be completed at this time.

Traffic signal warrants based on peak hour volumes as adopted by the Federal Highway Administration and Caltrans were used here to determine the need for signalization. In applying this warrant, the volumes of both the major and minor street must meet or exceed those shown on the curves in Figure 30 for conditions when the speed on the major street is 40 (mph) or higher which is expected to be experienced by Rancho Parkway.

Determining the major street approach for the signal warrant involves calculating the number of vehicles approaching the intersection on both major street legs. The highest total volume for either the continuous east and west approach or the north and south approach during either AM and PM is determined to be the major street approach for both peak hours. The minor street peak hour signal warrant volume is the number of peak hour vehicles approaching the intersection on only the highest volume leg. The highest volume for either the AM or PM determines the minor approach for both peak hours.

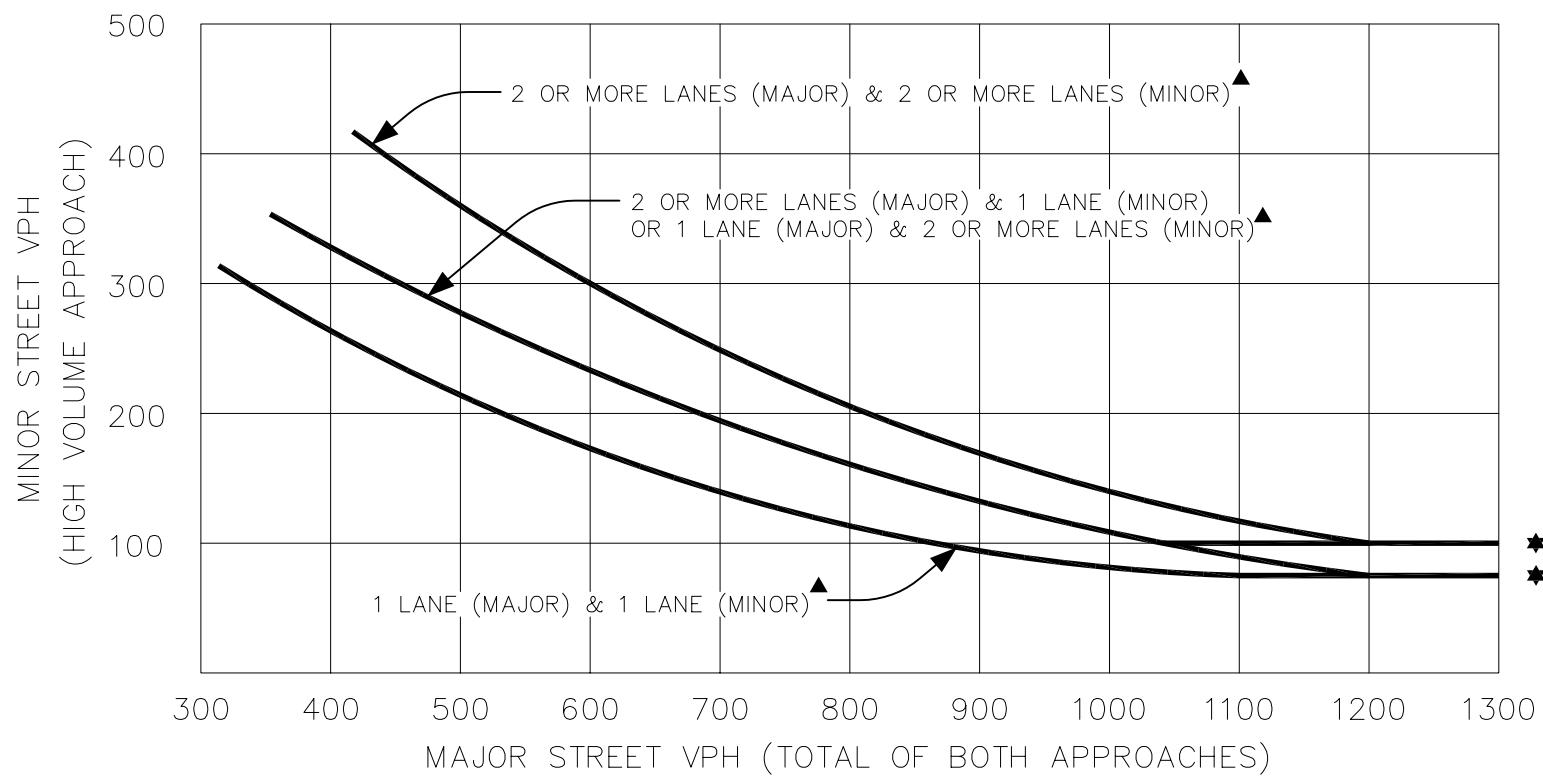
The signal warrant analysis has been carried out for the intersection of the main entrance to the proposed sports park on Rancho Parkway. Since the worst-case volume on the major street (Rancho

Table 14

SHORT-TERM (YEAR 2015 CUMULATIVE) FREEWAY/TOLLWAY MAINLINE LOS SUMMARY

Location	Direction	Lanes	Peak Hour Capacity	No-Project						With-Project					
				AM Peak Hour			PM Peak Hour			AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS									
I-5 n/o Lake Forest	Northbound	8+2H	19,500	15,508	.80	D	12,651	.65	C	15,508	.80	D	12,682	.65	C
	Southbound	8+2H	19,500	11,806	.61	C	15,990	.82	D	11,844	.61	C	15,996	.82	D
I-5 n/o El Toro	Northbound	6+2H	15,500	15,527	1.00	E	12,269	.79	D	15,553	1.00	E	12,275	.79	D
	Southbound	6+2H	15,500	10,546	.68	C	14,546	.94	E	10,559	.68	C	14,546	.94	E
I-5 n/o Alicia	Northbound	4+1H	9,600	14,414	1.50	F	11,439	1.19	F	14,414	1.50	F	11,454	1.19	F
	Southbound	4+1H	9,600	9,661	1.01	F	14,370	1.50	F	9,661	1.01	F	14,370	1.50	F
SR-241 n/o Lake Forest	Northbound	3	6,000	5,079	.85	D	2,419	.40	B	5,079	.85	D	2,419	.40	B
	Southbound	3	6,000	2,027	.34	B	4,522	.75	D	2,027	.34	B	4,533	.76	D
SR-241 n/o Portola East	Northbound	3	6,000	4,957	.83	D	1,836	.31	B	4,957	.83	D	1,836	.31	B
	Southbound	3	6,000	1,387	.23	A	4,265	.71	C	1,387	.23	A	4,265	.71	C
SR-241 n/o Los Alisos	Northbound	3	6,000	4,863	.81	D	1,636	.27	A	4,863	.81	D	1,636	.27	A
	Southbound	3	6,000	1,195	.20	A	4,022	.67	C	1,195	.20	A	4,022	.67	C

Abbreviations: H – high-occupancy vehicle lane
 LOS – level of service
 V/C – volume/capacity ratio



- ▲ NOTE: THESE CURVES ARE RECOMMENDED FOR USE IN AREAS WHERE THE POSTED SPEED LIMIT ON THE MAJOR STREET IS 40 MPH OR HIGHER.
- ▲ NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES, AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Figure 30

PEAK HOUR SIGNAL WARRANTS
(HIGHER SPEEDS)

Parkway) is forecast to be over 1,500 vehicles in the PM peak hour under year 2015 cumulative with-project conditions, only a minimum volume of 75 vehicles on the minor street (proposed project access driveway) is required to determine if signal warrants are met. This indicates that of the total forecast volume in the PM peak hour of 503 only 15 percent of vehicles arriving and leaving the sports park would meet signal warrants. It is likely that this will occur and based on the application of the warrant, traffic signals should be installed at the proposed access intersection at Rancho Parkway under year 2015 cumulative with-project conditions. However, signals typically are not installed until warrants are met.

FINDINGS AND CONCLUSIONS

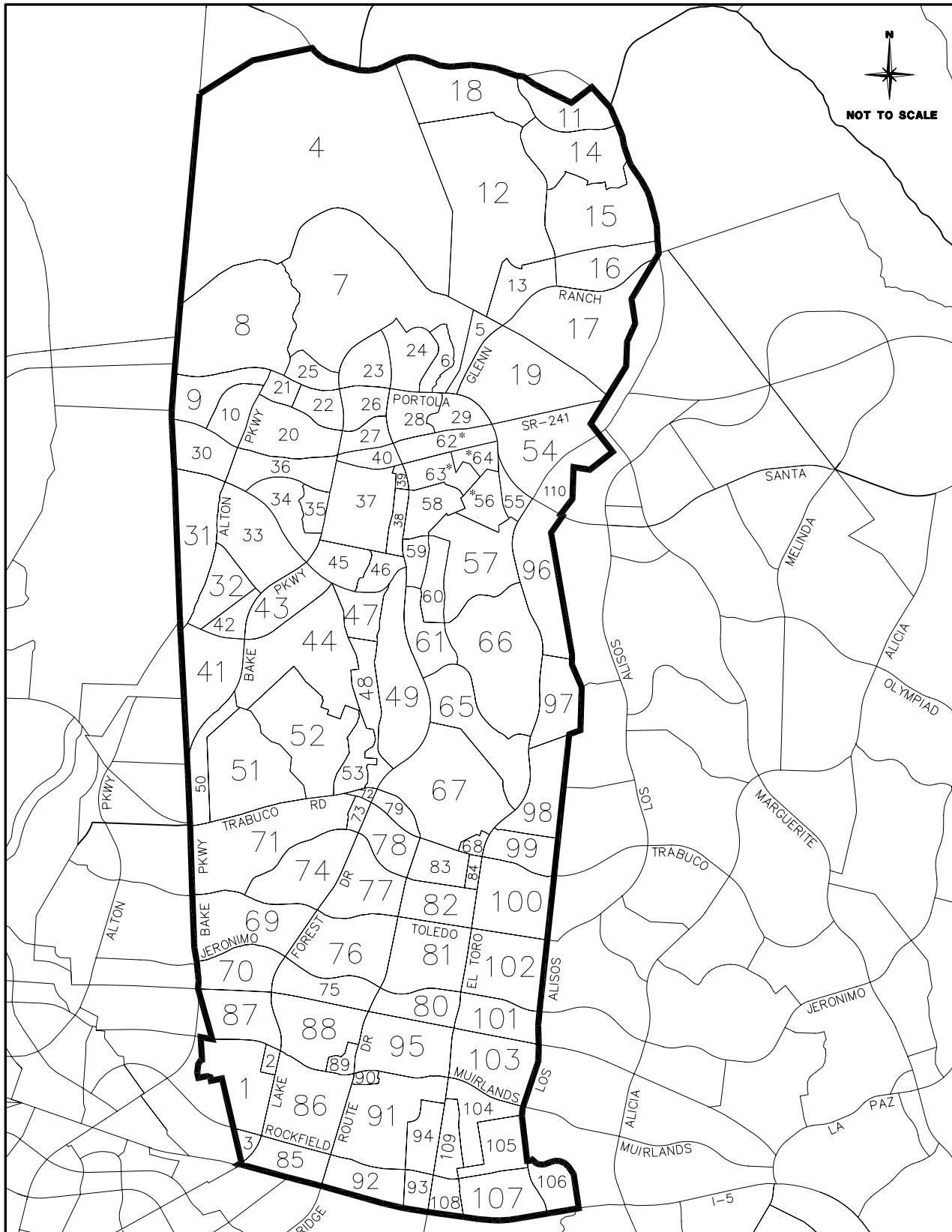
The proposed project involves the development of a sports park and recreation center oriented toward youth, adult, and senior recreation activities. The development of the proposed project has relatively modest impacts on the surrounding street system during the AM and PM peak hours. The results of the analysis presented here indicate that the proposed project does not adversely impact any locations with the exception of Lake Forest Drive and Rancho Parkway in year 2015 cumulative in the PM peak hour. The improvements for this location are included in the LFTM Program and according to the analysis presented in this report their implementation would be required sooner than later. These improvements are needed as well as the Rancho Parkway extension to Portola Parkway and new intersection to accommodate the proposed project.

REFERENCES

1. "City of Lake Forest Vacant Land Opportunities Phase III Traffic Study," Austin-Foust Associates, Inc., July 8, 2005.
2. "City of Lake Forest Vacant Land Opportunities Phase III Alternative 7 (Hybrid Alternative) Traffic Study," Austin-Foust Associates, Inc., November 7, 2007 (Approved by Lake Forest City Council on June 3, 2008).
3. "City of Lake Forest Vacant Land Opportunities Phase III Alternative 8 Traffic Study," Austin-Foust Associates, Inc., September 2009.

Appendix A

Land Use and Trip Generation



Legend

- City of Lake Forest boundary
- * Zones affected by project

Figure A-1

**LAKE FOREST TRAFFIC ANALYSIS MODEL
(LFTAM) ZONE SYSTEM
- CITY OF LAKE FOREST**

Table A-1

SHORT-TERM (YEAR 2011 AND YEAR 2015 CUMULATIVE) AFFECTED ZONES LAND USE AND TRIP GENERATION COMPARISON

Zone	Land Use Category	Units	---No-Project---		---With-Project---		--- Difference ---	
			Amount	ADT	Amount	ADT	Amount	ADT
56	36 Park	Acre	58.60	93	20.60	33	-38.00	-60
	46 Sports Park	Acre	--	--	38.00	2,044	38.00	2,044
	SUB-TOTAL			93		2,077		1,984
63	42 Business Park	TSF	435.72	5,560	435.72	5,560	0.00	0
	46 Sports Park	Acre	--	--	13.00	699	13.00	699
	47 Vacant	ACRE	13.20	0	--	--	-13.20	0
	SUB-TOTAL				5,560		6,259	699
64	41 Mining/Utility	Acre	16.10	401	--	--	-16.10	-401
	46 Sports Park	Acre	--	--	16.10	866	16.10	866
	SUB-TOTAL			401			866	465
TOTAL	36 Park	Acre	58.60	93	20.60	33	-38.00	-60
	41 Mining/Utility	Acre	16.10	401	--	--	-16.10	-401
	42 Business Park	TSF	435.72	5,560	435.72	5,560	0.00	0
	46 Sports Park	Acre	--	--	67.10	3,609	67.10	3,609
	47 Vacant	ACRE	13.20	0	--	--	-13.20	0
	TOTAL				6,054		9,202	3,148

Table A-2

SHORT-TERM (YEAR 2011 AND YEAR 2015 CUMULATIVE) AFFECTED ZONES ZONAL LAND USE AND TRIP GENERATION

Zone	Land Use Type	Units	-- AM Peak Hour --			-- PM Peak Hour --			ADT
			In	Out	Total	In	Out	Total	
NO-PROJECT									
56	36 Park SUB-TOTAL	58.60 Acre	1 1	0 0	1 1	1 1	1 1	2 2	93 93
63	42 Business Park 47 Vacant SUB-TOTAL	435.72 TSF 13.20 ACRE	523 0	100 0	623 0	131 0	431 0	562 0	5560 0
64	41 Mining/Utility SUB-TOTAL	16.10 Acre	25 25	15 15	40 40	9 9	12 12	21 21	401 401
TOTAL (NO-PROJECT)									
36	Park	58.60 Acre	1	0	1	1	1	2	93
41	Mining/Utility	16.10 Acre	25	15	40	9	12	21	401
42	Business Park	435.72 TSF	523	100	623	131	431	562	5560
47	Vacant	13.20 ACRE	0	0	0	0	0	0	0
TOTAL (NO-PROJECT)									
WITH-PROJECT									
56	36 Park 46 Sports Park SUB-TOTAL	20.60 Acre 38.00 Acre	0 0	0 0	0 0	0 129	0 156	0 285	33 2044
63	42 Business Park 46 Sports Park SUB-TOTAL	435.72 TSF 13.00 Acre	523 0	100 0	623 0	131 44	431 53	562 97	5560 699
64	46 Sports Park SUB-TOTAL	16.10 Acre	0 0	0 0	0 0	55 55	66 66	121 121	866 866
TOTAL (WITH-PROJECT)									
36	Park	20.60 Acre	0	0	0	0	0	0	33
42	Business Park	435.72 TSF	523	100	623	131	431	562	5560
46	Sports Park	67.10 Acre	0	0	0	228	275	503	3609
TOTAL (WITH-PROJECT)									
DIFFERENCE									
36 Park	Acre	.01	.00	.01	.02	.02	.04	1.59	
41 Mining/Utility	Acre	1.57	.92	2.49	.59	.73	1.32	24.90	
42 Business Park	TSF	1.20	.23	1.43	.30	.99	1.29	12.76	
46 Sports Park	Acre	.01	.00	.01	3.40	4.10	7.50	53.80	
47 Vacant	ACRE	.00	.00	.00	.00	.00	.00	.00	

Appendix B

Santa Margarita Parkway Count Sheets

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALISOS BLVD

VOLUMES FOR - TUESDAY 8/4/09

***** AM *****				***** PM *****			
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL
12:00 - 12:15	44	26	70	12:00 - 12:15	-	-	-
12:15 - 12:30	43	26	69	12:15 - 12:30	-	-	-
12:30 - 12:45	26	22	48	12:30 - 12:45	-	-	-
12:45 - 1:00	25	138	95	46	233	12:45 - 1:00	-
1:00 - 1:15	35	14	49	1:00 - 1:15	-	-	-
1:15 - 1:30	21	16	37	1:15 - 1:30	-	-	-
1:30 - 1:45	15	7	22	1:30 - 1:45	-	-	-
1:45 - 2:00	13	84	51	27	135	1:45 - 2:00	-
2:00 - 2:15	15	7	22	2:00 - 2:15	-	-	-
2:15 - 2:30	12	8	20	2:15 - 2:30	-	-	-
2:30 - 2:45	6	8	14	2:30 - 2:45	-	-	-
2:45 - 3:00	7	40	10	33	17	73	2:45 - 3:00
3:00 - 3:15	10	10	20	3:00 - 3:15	-	-	-
3:15 - 3:30	14	12	26	3:15 - 3:30	-	-	-
3:30 - 3:45	5	20	25	3:30 - 3:45	-	-	-
3:45 - 4:00	12	41	18	60	30	101	3:45 - 4:00
4:00 - 4:15	8	13	21	4:00 - 4:15	-	-	-
4:15 - 4:30	10	17	27	4:15 - 4:30	-	-	-
4:30 - 4:45	12	42	54	4:30 - 4:45	-	-	-
4:45 - 5:00	20	50	122	70	172	4:45 - 5:00	-
5:00 - 5:15	26	64	90	5:00 - 5:15	-	-	-
5:15 - 5:30	25	66	91	5:15 - 5:30	-	-	-
5:30 - 5:45	34	101	135	5:30 - 5:45	-	-	-
5:45 - 6:00	50	135	128	359	178	494	5:45 - 6:00
6:00 - 6:15	52	130	182	6:00 - 6:15	-	-	-
6:15 - 6:30	56	150	206	6:15 - 6:30	-	-	-
6:30 - 6:45	80	224	304	6:30 - 6:45	-	-	-
6:45 - 7:00	100	288	260	764	360	1052	6:45 - 7:00
7:00 - 7:15	90	298	388	7:00 - 7:15	-	-	-
7:15 - 7:30	118	367	485	7:15 - 7:30	-	-	-
7:30 - 7:45	124	329	453	7:30 - 7:45	-	-	-
7:45 - 8:00	159	491	389	1383	548	1874	7:45 - 8:00
8:00 - 8:15	150	331	481	8:00 - 8:15	-	-	-
8:15 - 8:30	133	371	504	8:15 - 8:30	-	-	-
8:30 - 8:45	132	334	466	8:30 - 8:45	-	-	-
8:45 - 9:00	182	597	327	1363	509	1960	8:45 - 9:00
9:00 - 9:15	184	303	487	9:00 - 9:15	-	-	-
9:15 - 9:30	120	284	404	9:15 - 9:30	-	-	-
9:30 - 9:45	134	272	406	9:30 - 9:45	-	-	-
9:45 - 10:00	146	584	238	1097	384	1681	9:45 - 10:00
10:00 - 10:15	-	-	-	10:00 - 10:15	-	-	-
10:15 - 10:30	-	-	-	10:15 - 10:30	-	-	-
10:30 - 10:45	-	-	-	10:30 - 10:45	-	-	-
10:45 - 11:00	-	-	-	10:45 - 11:00	-	-	-
11:00 - 11:15	-	-	-	11:00 - 11:15	-	-	-
11:15 - 11:30	-	-	-	11:15 - 11:30	-	-	-
11:30 - 11:45	-	-	-	11:30 - 11:45	-	-	-
11:45 - 12:00	-	-	-	11:45 - 12:00	-	-	-

TOTALS 2,448 5,327 7,775 - - -

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALIOS BLVD

VOLUMES FOR - THURSDAY 7/30/09

***** AM *****				***** PM *****			
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL
12:00 - 12:15	-	-	-	12:00 - 12:15	294	262	556
12:15 - 12:30	-	-	-	12:15 - 12:30	256	330	586
12:30 - 12:45	-	-	-	12:30 - 12:45	279	258	537
12:45 - 1:00	-	-	-	12:45 - 1:00	357	1186	2294
1:00 - 1:15	-	-	-	1:00 - 1:15	298	253	551
1:15 - 1:30	-	-	-	1:15 - 1:30	267	244	511
1:30 - 1:45	-	-	-	1:30 - 1:45	228	243	471
1:45 - 2:00	-	-	-	1:45 - 2:00	246	1039	502
2:00 - 2:15	-	-	-	2:00 - 2:15	250	248	498
2:15 - 2:30	-	-	-	2:15 - 2:30	242	239	481
2:30 - 2:45	-	-	-	2:30 - 2:45	266	275	541
2:45 - 3:00	-	-	-	2:45 - 3:00	244	1002	470
3:00 - 3:15	-	-	-	3:00 - 3:15	236	218	454
3:15 - 3:30	-	-	-	3:15 - 3:30	300	225	525
3:30 - 3:45	-	-	-	3:30 - 3:45	308	218	526
3:45 - 4:00	-	-	-	3:45 - 4:00	336	1180	2061
4:00 - 4:15	-	-	-	4:00 - 4:15	316	226	542
4:15 - 4:30	-	-	-	4:15 - 4:30	336	254	590
4:30 - 4:45	-	-	-	4:30 - 4:45	396	249	645
4:45 - 5:00	-	-	-	4:45 - 5:00	364	1412	2365
5:00 - 5:15	-	-	-	5:00 - 5:15	400	222	622
5:15 - 5:30	-	-	-	5:15 - 5:30	489	255	744
5:30 - 5:45	-	-	-	5:30 - 5:45	462	250	712
5:45 - 6:00	-	-	-	5:45 - 6:00	412	1763	2757
6:00 - 6:15	-	-	-	6:00 - 6:15	442	257	699
6:15 - 6:30	-	-	-	6:15 - 6:30	360	248	608
6:30 - 6:45	-	-	-	6:30 - 6:45	390	234	624
6:45 - 7:00	-	-	-	6:45 - 7:00	315	1507	2490
7:00 - 7:15	-	-	-	7:00 - 7:15	278	248	526
7:15 - 7:30	-	-	-	7:15 - 7:30	290	230	520
7:30 - 7:45	-	-	-	7:30 - 7:45	296	214	510
7:45 - 8:00	-	-	-	7:45 - 8:00	225	1089	2001
8:00 - 8:15	-	-	-	8:00 - 8:15	220	200	420
8:15 - 8:30	-	-	-	8:15 - 8:30	231	176	407
8:30 - 8:45	-	-	-	8:30 - 8:45	214	166	380
8:45 - 9:00	-	-	-	8:45 - 9:00	193	858	1542
9:00 - 9:15	-	-	-	9:00 - 9:15	193	144	337
9:15 - 9:30	-	-	-	9:15 - 9:30	216	134	350
9:30 - 9:45	-	-	-	9:30 - 9:45	236	114	350
9:45 - 10:00	-	-	-	9:45 - 10:00	164	809	1317
10:00 - 10:15	168	210	378	10:00 - 10:15	156	84	240
10:15 - 10:30	168	190	358	10:15 - 10:30	140	84	224
10:30 - 10:45	164	202	366	10:30 - 10:45	125	72	197
10:45 - 11:00	166	666	814	10:45 - 11:00	108	529	838
11:00 - 11:15	161	217	378	11:00 - 11:15	92	65	157
11:15 - 11:30	191	240	431	11:15 - 11:30	96	48	144
11:30 - 11:45	206	250	456	11:30 - 11:45	76	39	115
11:45 - 12:00	209	767	973	11:45 - 12:00	56	320	493

TOTALS 1,433 1,787 3,220 12,694 9,489 22,183

ADT'S

14,127 11,276 25,403

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALIOS BLVD

VOLUMES FOR - FRIDAY 7/31/09

***** AM *****						***** PM *****					
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL				
12:00 - 12:15	50	39	89	12:00 - 12:15	234	262	496				
12:15 - 12:30	38	20	58	12:15 - 12:30	270	315	585				
12:30 - 12:45	41	24	65	12:30 - 12:45	238	281	519				
12:45 - 1:00	26	155	99	42	254	1104	290	1148	652	2252	
1:00 - 1:15	36	31	67	1:00 - 1:15	283	294	577				
1:15 - 1:30	36	19	55	1:15 - 1:30	250	263	513				
1:30 - 1:45	24	14	38	1:30 - 1:45	260	258	518				
1:45 - 2:00	17	113	9	73	26	186	1:45 - 2:00	238	1031	227	1042
2:00 - 2:15	22	16	38	2:00 - 2:15	270	272	542				
2:15 - 2:30	10	10	20	2:15 - 2:30	268	253	521				
2:30 - 2:45	20	10	30	2:30 - 2:45	276	259	535				
2:45 - 3:00	19	71	7	43	26	114	2:45 - 3:00	282	1096	244	1028
3:00 - 3:15	24	14	38	3:00 - 3:15	293	238	531				
3:15 - 3:30	12	10	22	3:15 - 3:30	286	244	530				
3:30 - 3:45	11	14	25	3:30 - 3:45	324	240	564				
3:45 - 4:00	16	63	10	48	26	111	3:45 - 4:00	341	1244	190	912
4:00 - 4:15	4	19	23	4:00 - 4:15	362	284	646				
4:15 - 4:30	12	20	32	4:15 - 4:30	357	247	604				
4:30 - 4:45	20	20	40	4:30 - 4:45	382	250	632				
4:45 - 5:00	18	54	52	111	70	165	4:45 - 5:00	420	1521	242	1023
5:00 - 5:15	22	43	65	5:00 - 5:15	439	259	698				
5:15 - 5:30	28	62	90	5:15 - 5:30	463	290	753				
5:30 - 5:45	44	92	136	5:30 - 5:45	480	270	750				
5:45 - 6:00	50	144	114	311	164	455	5:45 - 6:00	444	1826	236	1055
6:00 - 6:15	54	120	174	6:00 - 6:15	400	263	663				
6:15 - 6:30	60	140	200	6:15 - 6:30	334	272	606				
6:30 - 6:45	88	206	294	6:30 - 6:45	342	274	616				
6:45 - 7:00	105	307	237	703	342	1010	6:45 - 7:00	304	1380	238	1047
7:00 - 7:15	124	262	386	7:00 - 7:15	280	258	538				
7:15 - 7:30	130	335	465	7:15 - 7:30	280	200	480				
7:30 - 7:45	136	318	454	7:30 - 7:45	224	232	456				
7:45 - 8:00	162	552	390	1305	552	1857	7:45 - 8:00	243	1027	202	892
8:00 - 8:15	154	358	512	8:00 - 8:15	218	208	426				
8:15 - 8:30	156	399	555	8:15 - 8:30	209	196	405				
8:30 - 8:45	158	380	538	8:30 - 8:45	227	170	397				
8:45 - 9:00	194	662	331	1468	525	2130	8:45 - 9:00	227	881	142	716
9:00 - 9:15	213	249	462	9:00 - 9:15	164	142	306				
9:15 - 9:30	152	272	424	9:15 - 9:30	197	138	335				
9:30 - 9:45	211	230	441	9:30 - 9:45	198	138	336				
9:45 - 10:00	156	732	231	982	387	1714	9:45 - 10:00	164	723	116	534
10:00 - 10:15	176	208	384	10:00 - 10:15	164	112	276				
10:15 - 10:30	174	236	410	10:15 - 10:30	145	110	255				
10:30 - 10:45	208	220	428	10:30 - 10:45	134	92	226				
10:45 - 11:00	168	726	246	910	414	1636	10:45 - 11:00	102	545	94	408
11:00 - 11:15	221	252	473	11:00 - 11:15	112	76	188				
11:15 - 11:30	202	232	434	11:15 - 11:30	116	76	192				
11:30 - 11:45	206	264	470	11:30 - 11:45	121	53	174				
11:45 - 12:00	266	895	247	995	513	1890	11:45 - 12:00	96	445	57	262
TOTALS	4,474	7,048	11,522				12,823	10,067	22,890		

ADT'S

17,297 17,115 34,412

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALISOS BLVD

VOLUMES FOR - SATURDAY 8/1/09

***** AM *****				***** PM *****			
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL
12:00 - 12:15	60	44	104	12:00 - 12:15	260	299	559
12:15 - 12:30	52	48	100	12:15 - 12:30	272	304	576
12:30 - 12:45	61	28	89	12:30 - 12:45	232	249	481
12:45 - 1:00	50	223	148	78	371	1020	266
				12:45 - 1:00	256	1118	522
1:00 - 1:15	48	19	67	1:00 - 1:15	238	297	535
1:15 - 1:30	55	16	71	1:15 - 1:30	264	296	560
1:30 - 1:45	42	30	72	1:30 - 1:45	270	216	486
1:45 - 2:00	27	172	87	49	259	1007	256
				1:45 - 2:00	235	1065	491
2:00 - 2:15	32	14	46	2:00 - 2:15	275	245	520
2:15 - 2:30	26	17	43	2:15 - 2:30	252	248	500
2:30 - 2:45	20	21	41	2:30 - 2:45	266	244	510
2:45 - 3:00	18	96	13	65	31	161	271
				2:45 - 3:00	1064	214	951
3:00 - 3:15	18	6	24	3:00 - 3:15	225	217	442
3:15 - 3:30	22	15	37	3:15 - 3:30	252	213	465
3:30 - 3:45	8	14	22	3:30 - 3:45	265	220	485
3:45 - 4:00	17	65	11	46	28	111	254
				3:45 - 4:00	996	246	896
4:00 - 4:15	9	10	19	4:00 - 4:15	239	260	499
4:15 - 4:30	16	16	32	4:15 - 4:30	240	239	479
4:30 - 4:45	16	20	36	4:30 - 4:45	217	220	437
4:45 - 5:00	13	54	20	66	33	120	228
				4:45 - 5:00	924	225	944
5:00 - 5:15	18	19	37	5:00 - 5:15	241	232	473
5:15 - 5:30	16	22	38	5:15 - 5:30	206	246	452
5:30 - 5:45	29	38	67	5:30 - 5:45	218	232	450
5:45 - 6:00	32	95	48	127	80	222	207
				5:45 - 6:00	872	246	956
6:00 - 6:15	35	38	73	6:00 - 6:15	244	222	466
6:15 - 6:30	24	60	84	6:15 - 6:30	240	271	511
6:30 - 6:45	60	76	136	6:30 - 6:45	217	232	449
6:45 - 7:00	57	176	102	276	159	452	218
				6:45 - 7:00	919	198	923
7:00 - 7:15	80	102	182	7:00 - 7:15	190	186	376
7:15 - 7:30	83	100	183	7:15 - 7:30	243	184	427
7:30 - 7:45	72	102	174	7:30 - 7:45	170	185	355
7:45 - 8:00	106	341	152	456	258	797	217
				7:45 - 8:00	820	175	730
8:00 - 8:15	98	136	234	8:00 - 8:15	221	161	382
8:15 - 8:30	120	138	258	8:15 - 8:30	228	181	409
8:30 - 8:45	118	170	288	8:30 - 8:45	196	152	348
8:45 - 9:00	148	484	187	631	335	1115	166
				8:45 - 9:00	811	139	633
9:00 - 9:15	149	167	316	9:00 - 9:15	197	140	337
9:15 - 9:30	163	186	349	9:15 - 9:30	180	133	313
9:30 - 9:45	159	208	367	9:30 - 9:45	160	132	292
9:45 - 10:00	188	659	218	779	406	1438	9:45 - 10:00
				158	695	92	497
10:00 - 10:15	168	205	373	10:00 - 10:15	138	116	254
10:15 - 10:30	191	244	435	10:15 - 10:30	156	103	259
10:30 - 10:45	194	263	457	10:30 - 10:45	126	66	192
10:45 - 11:00	219	772	248	960	467	1732	10:45 - 11:00
				92	512	66	351
11:00 - 11:15	180	274	454	11:00 - 11:15	102	76	178
11:15 - 11:30	221	254	475	11:15 - 11:30	98	49	147
11:30 - 11:45	223	245	468	11:30 - 11:45	102	42	144
11:45 - 12:00	232	856	324	1097	556	1953	11:45 - 12:00
				81	383	40	207
TOTALS	3,993	4,738	8,731		10,023	9,271	19,294

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALISOS BLVD

VOLUMES FOR - SUNDAY 8/2/09

***** AM *****				***** PM *****				
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL	
12:00 - 12:15	78	48	126	12:00 - 12:15	227	214	441	
12:15 - 12:30	48	44	92	12:15 - 12:30	200	244	444	
12:30 - 12:45	50	40	90	12:30 - 12:45	228	242	470	
12:45 - 1:00	55	231	30 162	85 393	12:45 - 1:00	300 955	226 926	526 1881
1:00 - 1:15	48	30	78	1:00 - 1:15	358	260	618	
1:15 - 1:30	50	23	73	1:15 - 1:30	278	242	520	
1:30 - 1:45	46	21	67	1:30 - 1:45	234	246	480	
1:45 - 2:00	27	171	21 95	48 266	1:45 - 2:00	258 1128	259 1007	517 2135
2:00 - 2:15	22	20	42	2:00 - 2:15	238	276	514	
2:15 - 2:30	18	8	26	2:15 - 2:30	251	207	458	
2:30 - 2:45	29	16	45	2:30 - 2:45	260	216	476	
2:45 - 3:00	25	94	11 55	36 149	2:45 - 3:00	256 1005	242 941	498 1946
3:00 - 3:15	15	13	28	3:00 - 3:15	232	200	432	
3:15 - 3:30	6	9	15	3:15 - 3:30	239	180	419	
3:30 - 3:45	9	22	31	3:30 - 3:45	222	228	450	
3:45 - 4:00	15	45	11 55	26 100	3:45 - 4:00	238 931	206 814	444 1745
4:00 - 4:15	8	9	17	4:00 - 4:15	222	238	460	
4:15 - 4:30	8	8	16	4:15 - 4:30	212	245	457	
4:30 - 4:45	9	16	25	4:30 - 4:45	206	207	413	
4:45 - 5:00	10	35	8 41	18 76	4:45 - 5:00	234 874	203 893	437 1767
5:00 - 5:15	19	20	39	5:00 - 5:15	218	196	414	
5:15 - 5:30	13	19	32	5:15 - 5:30	219	206	425	
5:30 - 5:45	19	20	39	5:30 - 5:45	208	192	400	
5:45 - 6:00	26	77	25 84	51 161	5:45 - 6:00	215 860	184 778	399 1638
6:00 - 6:15	22	23	45	6:00 - 6:15	191	204	395	
6:15 - 6:30	26	44	70	6:15 - 6:30	214	220	434	
6:30 - 6:45	44	48	92	6:30 - 6:45	196	186	382	
6:45 - 7:00	40	132	61 176	101 308	6:45 - 7:00	194 795	180 790	374 1585
7:00 - 7:15	45	66	111	7:00 - 7:15	204	152	356	
7:15 - 7:30	44	73	117	7:15 - 7:30	209	158	367	
7:30 - 7:45	59	88	147	7:30 - 7:45	164	149	313	
7:45 - 8:00	74	222	120 347	194 569	7:45 - 8:00	172 749	138 597	310 1346
8:00 - 8:15	72	96	168	8:00 - 8:15	168	168	336	
8:15 - 8:30	78	115	193	8:15 - 8:30	173	182	355	
8:30 - 8:45	72	162	234	8:30 - 8:45	144	129	273	
8:45 - 9:00	86	308	209 582	295 890	8:45 - 9:00	154 639	134 613	288 1252
9:00 - 9:15	92	176	268	9:00 - 9:15	154	132	286	
9:15 - 9:30	96	160	256	9:15 - 9:30	136	131	267	
9:30 - 9:45	106	144	250	9:30 - 9:45	166	82	248	
9:45 - 10:00	146	440	166 646	312 1086	9:45 - 10:00	188 644	112 457	300 1101
10:00 - 10:15	131	175	306	10:00 - 10:15	177	88	265	
10:15 - 10:30	164	205	369	10:15 - 10:30	117	62	179	
10:30 - 10:45	219	200	419	10:30 - 10:45	92	41	133	
10:45 - 11:00	206	720	253 833	459 1553	10:45 - 11:00	100 486	37 228	137 714
11:00 - 11:15	186	294	480	11:00 - 11:15	76	46	122	
11:15 - 11:30	180	282	462	11:15 - 11:30	78	51	129	
11:30 - 11:45	185	226	411	11:30 - 11:45	72	34	106	
11:45 - 12:00	188	739	248 1050	436 1789	11:45 - 12:00	48 274	39 170	87 444
TOTALS	3,214	4,126	7,340		9,340	8,214	17,554	
ADT'S					12,554	12,340	24,894	

TRAFFIC DATA SERVICES, INC.

LOCATION CODE 07908.001

LOCATION - SANTA MARGARITA PKWY-BTN EL TORO/LOS ALTSOS BLVD

VOLUMES FOR - MONDAY 8/3/09

AM				PM					
TIME	EB	WB	TOTAL	TIME	EB	WB	TOTAL		
12:00 - 12:15	40	37	77	12:00 - 12:15	250	216	466		
12:15 - 12:30	43	20	63	12:15 - 12:30	218	220	438		
12:30 - 12:45	27	16	43	12:30 - 12:45	188	251	439		
12:45 - 1:00	22	132	17 90	39 222	12:45 - 1:00	207	863	244 931	451 1794
1:00 - 1:15	42	27	69	1:00 - 1:15	216	245	461		
1:15 - 1:30	28	8	36	1:15 - 1:30	218	231	449		
1:30 - 1:45	19	6	25	1:30 - 1:45	224	208	432		
1:45 - 2:00	18	107	14 55	32 162	1:45 - 2:00	234	892	192 876	426 1768
2:00 - 2:15	10	9	19	2:00 - 2:15	240	200	440		
2:15 - 2:30	9	10	19	2:15 - 2:30	206	206	412		
2:30 - 2:45	7	6	13	2:30 - 2:45	224	206	430		
2:45 - 3:00	11	37	8 33	19 70	2:45 - 3:00	253	923	233 845	486 1768
3:00 - 3:15	12	12	24	3:00 - 3:15	251	208	459		
3:15 - 3:30	13	14	27	3:15 - 3:30	255	212	467		
3:30 - 3:45	8	13	21	3:30 - 3:45	284	204	488		
3:45 - 4:00	13	46	17 56	30 102	3:45 - 4:00	284	1074	205 829	489 1903
4:00 - 4:15	5	12	17	4:00 - 4:15	306	224	530		
4:15 - 4:30	12	22	34	4:15 - 4:30	340	225	565		
4:30 - 4:45	20	30	50	4:30 - 4:45	344	228	572		
4:45 - 5:00	19	56	44 108	63 164	4:45 - 5:00	392	1382	233 910	625 2292
5:00 - 5:15	20	64	84	5:00 - 5:15	412	253	665		
5:15 - 5:30	25	62	87	5:15 - 5:30	517	258	775		
5:30 - 5:45	34	97	131	5:30 - 5:45	448	256	704		
5:45 - 6:00	49	128	112 335	161 463	5:45 - 6:00	428	1805	230 997	658 2802
6:00 - 6:15	50	122	172	6:00 - 6:15	436	229	665		
6:15 - 6:30	38	154	192	6:15 - 6:30	378	242	620		
6:30 - 6:45	76	208	284	6:30 - 6:45	333	222	555		
6:45 - 7:00	85	249	263 747	348 996	6:45 - 7:00	286	1433	231 924	517 2357
7:00 - 7:15	90	309	399	7:00 - 7:15	259	196	455		
7:15 - 7:30	132	328	460	7:15 - 7:30	270	181	451		
7:30 - 7:45	110	318	428	7:30 - 7:45	244	191	435		
7:45 - 8:00	142	474	356 1311	498 1785	7:45 - 8:00	200	973	180 748	380 1721
8:00 - 8:15	139	380	519	8:00 - 8:15	214	145	359		
8:15 - 8:30	128	348	476	8:15 - 8:30	202	156	358		
8:30 - 8:45	156	323	479	8:30 - 8:45	178	140	318		
8:45 - 9:00	164	587	310 1361	474 1948	8:45 - 9:00	160	754	109 550	269 1304
9:00 - 9:15	170	268	438	9:00 - 9:15	192	133	325		
9:15 - 9:30	136	280	416	9:15 - 9:30	182	102	284		
9:30 - 9:45	132	260	392	9:30 - 9:45	149	101	250		
9:45 - 10:00	182	620	200 1008	382 1628	9:45 - 10:00	140	663	103 439	243 1102
10:00 - 10:15	139	172	311	10:00 - 10:15	114	106	220		
10:15 - 10:30	150	212	362	10:15 - 10:30	116	78	194		
10:30 - 10:45	140	212	352	10:30 - 10:45	112	64	176		
10:45 - 11:00	178	607	184 780	362 1387	10:45 - 11:00	72	414	50 298	122 712
11:00 - 11:15	183	216	399	11:00 - 11:15	86	48	134		
11:15 - 11:30	178	231	409	11:15 - 11:30	73	36	109		
11:30 - 11:45	216	251	467	11:30 - 11:45	88	39	127		
11:45 - 12:00	202	779	210 908	412 1687	11:45 - 12:00	36	283	27 150	63 433
TOTALS	3,822	6,792	10,614		11,459	8,497	19,956		
ADT'S					15,281	15,289	30,570		

Appendix C

Intersection Capacity Utilization (ICU) Worksheets

This appendix summarizes information pertaining to the intersection analysis sections of the study.

ICU Calculation Methodology

The ICU calculation procedure is based on a critical movement methodology that shows the amount of capacity utilized by each critical movement at an intersection. A capacity of 1,700 vehicles per hour per lane is assumed together with a .05 clearance interval. A “de facto” right-turn lane is used in the ICU calculation for cases where a curb lane is wide enough to separately serve both through and right-turn traffic (typically with a width of 19 feet or more from curb to outside of through-lane with parking prohibited during peak periods). Such lanes are treated the same as striped right-turn lanes during the ICU calculations, but they are denoted on the ICU calculation worksheets using the letter “d” in place of a numerical entry for right-turn lanes.

The methodology also incorporates a check for right-turn capacity utilization. Both right-turn-on-green (RTOG) and right-turn-on-red (RTOR) capacity availability are calculated and checked against the total right-turn capacity need. If insufficient capacity is available, then an adjustment is made to the total capacity utilization value. The following example shows how this adjustment is made.

Example for Northbound Right

1. Right-Turn-On-Green (RTOG)

If NBT is critical move, then:

$$\text{RTOG} = \text{V/C (NBT)}$$

Otherwise,

$$\text{RTOG} = \text{V/C (NBL)} + \text{V/C (SBT)} - \text{V/C (SBL)}$$

2. Right-Turn-On-Red (RTOR)

If WBL is critical move, then:

$$\text{RTOR} = \text{V/C (WBL)}$$

Otherwise,

$$\text{RTOR} = \text{V/C (EBL)} + \text{V/C (WBT)} - \text{V/C (EBT)}$$

3. Right-Turn Overlap Adjustment

If the northbound right is assumed to overlap with the adjacent westbound left, adjustments to the RTOG and RTOR values are made as follows:

$$\text{RTOG} = \text{RTOG} + \text{V/C (WBL)}$$

$$\text{RTOR} = \text{RTOR} - \text{V/C (WBL)}$$

4. Total Right-Turn Capacity (RTC) Availability For NBR

$$\text{RTC} = \text{RTOG} + \text{factor} \times \text{RTOR}$$

Where factor = RTOR saturation flow factor (0% for County intersections, 75% for intersections in all other jurisdictions within the study area)

Right-turn adjustment is then as follows: Additional ICU = V/C (NBR) – RTC

A zero or negative value indicates that adequate capacity is available and no adjustment is necessary. A positive value indicates that the available RTOR and RTOG capacity does not adequately accommodate the right-turn V/C, therefore the right-turn is essentially considered to be a critical movement. In such cases, the right-turn adjustment is noted on the ICU worksheet and it is included in the total capacity utilization value. When it is determined that a right-turn adjustment is required for more than one right-turn movement, the word “multi” is printed on the worksheet instead of an actual right-turn movement reference, and the right-turn adjustments are cumulatively added to the total capacity utilization value. In such cases, further operational evaluation is typically carried out to determine if under actual operational conditions, the critical right-turns would operate simultaneously, and therefore a right-turn adjustment credit should be applied.

Shared Lane V/C Methodology

For intersection approaches where shared usage of a lane is permitted by more than one turn movement (e.g., left/through, through/right, left/through/right), the individual turn volumes are evaluated to determine whether dedication of the shared lane is warranted to any one given turn movement. The following example demonstrates how this evaluation is carried out:

Example for Shared Left/Through Lane

1. Average Lane Volume (ALV)

$$\text{ALV} = \frac{\text{Left-Turn Volume} + \text{Through Volume}}{\text{Total Left} + \text{Through Approach Lanes (including shared lane)}}$$

2. ALV for Each Approach

$$ALV \text{ (Left)} = \frac{\text{Left-Turn Volume}}{\text{Left Approach Lanes (including shared lane)}}$$

$$ALV \text{ (Through)} = \frac{\text{Through Volume}}{\text{Through Approach Lanes (including shared lane)}}$$

3. Lane Dedication is Warranted

If ALV (Left) is greater than ALV then full dedication of the shared lane to the left-turn approach is warranted. Left-turn and through V/C ratios for this case are calculated as follows:

$$V/C \text{ (Left)} = \frac{\text{Left-Turn Volume}}{\text{Left Approach Capacity (including shared lane)}}$$

$$V/C \text{ (Through)} = \frac{\text{Through Volume}}{\text{Through Approach Capacity (excluding shared lane)}}$$

Similarly, if ALV (Through) is greater than ALV then full dedication to the through approach is warranted, and left-turn and through V/C ratios are calculated as follows:

$$V/C \text{ (Left)} = \frac{\text{Left-Turn Volume}}{\text{Left Approach Capacity (excluding shared lane)}}$$

$$V/C \text{ (Through)} = \frac{\text{Through Volume}}{\text{Through Approach Capacity (including shared lane)}}$$

4. Lane Dedication is not Warranted

If ALV (Left) and ALV (Through) are both less than ALV, the left/through lane is assumed to be truly shared and each left, left/through or through approach lane carries an evenly distributed volume of traffic equal to ALV. A combined left/through V/C ratio is calculated as follows:

$$V/C \text{ (Left/Through)} = \frac{\text{Left-Turn Volume} + \text{Through Volume}}{\text{Total Left + Through Approach Capacity (including shared lane)}}$$

This V/C (Left/Through) ratio is assigned as the V/C (Through) ratio for the critical movement analysis and ICU summary listing.

If split phasing has not been designated for this approach, the relative proportion of V/C (Through) that is attributed to the left-turn volume is estimated as follows:

If approach has more than one left-turn (including shared lane), then:

$$V/C \text{ (Left)} = V/C \text{ (Through)}$$

If approach has only one left-turn lane (shared lane), then:

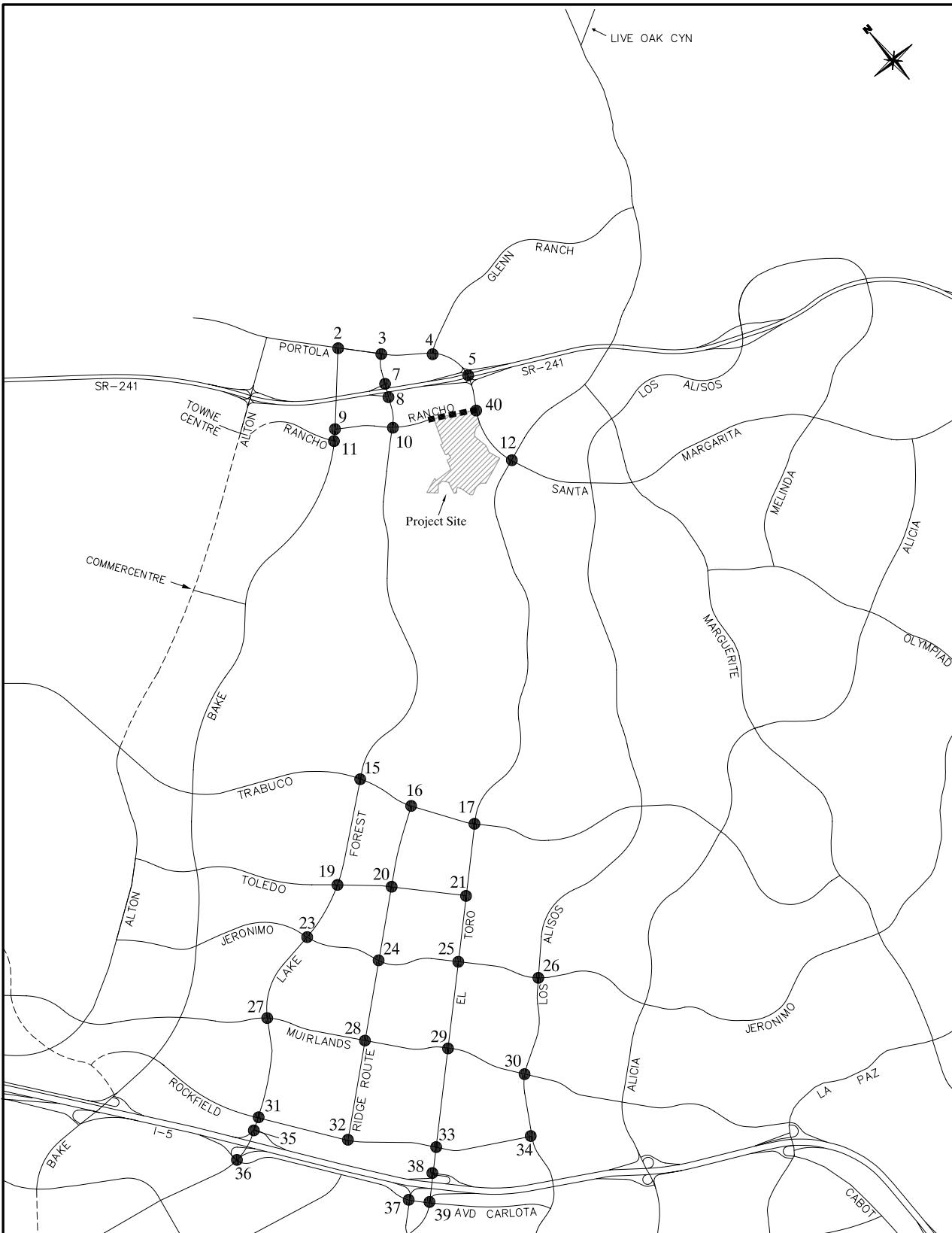
$$V/C \text{ (Left)} = \frac{\text{Left-Turn Volume}}{\text{Single Approach Lane Capacity}}$$

If this left-turn movement is determined to be a critical movement, the V/C (Left) value is posted in brackets on the ICU summary printout.

These same steps are carried out for shared through/right lanes. If full dedication of a shared through/right lane to the right-turn movement is warranted, the right-turn V/C value calculated in step three is checked against the RTOR and RTOG capacity. When an approach contains more than one shared lane (e.g., left/through and through/right), steps one and two listed above are carried out for the three turn movements combined. Step four is carried out if dedication is not warranted for either of the shared lanes. If dedication of one of the shared lanes is warranted to one movement or another, step three is carried out for the two movements involved, and then steps one through four are repeated for the two movements involved in the other shared lane.

Figure C-1 illustrates the intersections that were analyzed in this study. This is followed by AM and PM peak hour intersection capacity utilization (ICU) worksheets for two future short-term traffic conditions (year 2011 and year 2015 cumulative). The ICU data set is presented according to intersection number and contains the following scenarios in the order shown:

- Existing Count
- Short-Term (Year 2011) No-Project
- Short-Term (Year 2011) With-Project
- Short-Term (Year 2015 Cumulative) No-Project
- Short-Term (Year 2015 Cumulative) With-Project
- Short-Term (Year 2015 Cumulative) With-Project and Improvements



Legend

- Intersection location
- Future roadway in with-project only
- - - - Future roadway in by 2015

Figure C-1

SHORT-TERM (YEAR 2015 CUMULATIVE)
INTERSECTION LOCATION MAP

2. Bake & Portola

Existing Counts						2011 No-Project								
	LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C		LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C	
NBL	1	1700	68	.04*	288	.17*		NBL	1	1700	72	.04*	305	.18*
NBT	1.5	5100	143	{.04}	317	{.11}		NBT	1.5	5100	152	{.04}	336	{.11}
NBR	1.5		188		498			NBR	1.5		199		528	
SBL	1	1700	127	.07	206	.12		SBL	1	1700	135	.08	218	.13
SBT	2	3400	417	.12*	310	.09*		SBT	2	3400	442	.13*	329	.10*
SBR	d	1700	239	.14	171	.10		SBR	d	1700	253	.15	181	.11
EBL	1	1700	275	.16	244	.14*		EBL	1	1700	291	.17	259	.15*
EBT	3	5100	232	.05*	501	.10		EBT	3	5100	246	.05*	531	.10
EBR	d	1700	257	.15	128	.08		EBR	d	1700	272	.16	136	.08
WBL	2	3400	758	.22*	418	.12		WBL	2	3400	803	.24*	443	.13
WBT	2	3400	307	.09	542	.16*		WBT	2	3400	325	.10	575	.17*
WBR	d	1700	46	.03	63	.04		WBR	d	1700	49	.03	67	.04
Right Turn Adjustment		EBR	.07*				Right Turn Adjustment		EBR	.08*				
Clearance Interval			.05*				Clearance Interval			.05*			.05*	
TOTAL CAPACITY UTILIZATION			.55		.61		TOTAL CAPACITY UTILIZATION			.59		.65		

2011 With-Project						2015 No-Project									
	LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C		LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C		
NBL	1	1700	71	.04*	290	.17*		NBL	1	1700	130	.08*	330	.19	
NBT	1.5	5100	146	{.04}	334	{.11}		NBT	1.5	5100	130	{.04}	330	{.17}* <td></td>	
NBR	1.5		137		472			NBR	1.5		140		930		
SBL	1	1700	136	.08	222	.13		SBL	1	1700	130	.08	260	.15*	
SBT	2	3400	423	.12*	333	.10*		SBT	2	3400	290	.09*	310	.09	
SBR	d	1700	238	.14	175	.10		SBR	d	1700	230	.14	330	.19	
EBL	1	1700	290	.17*	245	.14*		EBL	1	1700	300	.18	370	.22*	
EBT	3	5100	267	.05	542	.11		EBT	3	5100	290	.06*	700	.14	
EBR	d	1700	282	.17	133	.08		EBR	d	1700	90	.05	140	.08	
WBL	2	3400	714	.21	377	.11		WBL	2	3400	1070	.31*	700	.21	
WBT	2	3400	335	.10*	593	.17*		WBT	2	3400	600	.18	770	.23*	
WBR	d	1700	50	.03	67	.04		WBR	d	1700	60	.04	160	.09	
Right Turn Adjustment		EBR	.08*				Clearance Interval			.05*		.05*			
Clearance Interval			.05*												
TOTAL CAPACITY UTILIZATION			.56		.63		TOTAL CAPACITY UTILIZATION			.59		.82			

2. Bake & Portola

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	120	.07*	330	.19
NBT	1.5	5100	130	{.04}	320	{.15}* 870
NBR	1.5		100			
SBL	1	1700	130	.08	260	.15*
SBT	2	3400	290	.09*	310	.09
SBR	d	1700	230	.14	320	.19
EBL	1	1700	300	.18*	370	.22*
EBT	3	5100	290	.06	710	.14
EBR	d	1700	90	.05	140	.08
WBL	2	3400	940	.28	600	.18
WBT	2	3400	600	.18*	800	.24*
WBR	d	1700	70	.04	160	.09
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.57		.81	

3. Lake Forest & Portola

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	30	.02	94	.06	NBL	1	1700	32	.02	100	.06
NBT	2	3400	151	.04*	192	.06*	NBT	2	3400	160	.05*	204	.06*
NBR	d	1700	185	.11	425	.25	NBR	d	1700	196	.12	450	.26
SBL	1	1700	207	.12*	180	.11*	SBL	1	1700	219	.13*	191	.11*
SBT	2	3400	192	.06	147	.04	SBT	2	3400	204	.06	156	.05
SBR	d	1700	6	.00	11	.01	SBR	d	1700	6	.00	12	.01
EBL	2	3400	21	.01	11	.00	EBL	2	3400	22	.01	12	.00
EBT	3	5100	447	.09*	1091	.21*	EBT	3	5100	474	.09*	1156	.23*
EBR	d	1700	49	.03	80	.05	EBR	d	1700	52	.03	85	.05
WBL	2	3400	559	.16*	363	.11*	WBL	2	3400	593	.17*	385	.11*
WBT	3	5100	1076	.21	901	.18	WBT	3	5100	1141	.22	955	.19
WBR	d	1700	266	.16	169	.10	WBR	d	1700	282	.17	179	.11
Right Turn Adjustment				NBR	.11*		Right Turn Adjustment		NBR	.12*			
Clearance Interval			.05*		.05*		Clearance Interval		.05*		.05*		
TOTAL CAPACITY UTILIZATION			.46		.65		TOTAL CAPACITY UTILIZATION		.49		.68		

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	38	.02	104	.06	NBL	1	1700	30	.02	70	.04
NBT	2	3400	152	.04*	225	.07*	NBT	2	3400	100	.03*	70	.02*
NBR	d	1700	130	.08	266	.16	NBR	d	1700	360	.21	780	.46
SBL	1	1700	228	.13*	189	.11*	SBL	1	1700	250	.15*	240	.14*
SBT	2	3400	229	.07	164	.05	SBT	2	3400	70	.02	120	.04
SBR	d	1700	9	.01	12	.01	SBR	d	1700	10	.01	10	.01
EBL	2	3400	25	.01*	17	.01	EBL	2	3400	10	.00	10	.00
EBT	3	5100	422	.08	1087	.21*	EBT	3	5100	560	.11*	1520	.30*
EBR	d	1700	61	.04	101	.06	EBR	d	1700	30	.02	30	.02
WBL	2	3400	399	.12	333	.10*	WBL	2	3400	900	.26*	590	.17*
WBT	3	5100	1054	.21*	896	.18	WBT	3	5100	1830	.36	1190	.23
WBR	d	1700	276	.16	174	.10	WBR	d	1700	270	.16	190	.11
Right Turn Adjustment				NBR	.01*		Right Turn Adjustment		NBR	.31*			
Clearance Interval			.05*		.05*		Clearance Interval		.05*		.05*		
TOTAL CAPACITY UTILIZATION			.44		.55		TOTAL CAPACITY UTILIZATION		.60		.99		

3. Lake Forest & Portola

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	40	.02	100	.06
NBT	2	3400	120	.04*	100	.03*
NBR	d	1700	240	.14	470	.28
SBL	1	1700	230	.14*	220	.13*
SBT	2	3400	90	.03	140	.04
SBR	d	1700	10	.01	10	.01
EBL	2	3400	20	.01*	10	.00
EBT	3	5100	510	.10	1470	.29*
EBR	d	1700	50	.03	40	.02
WBL	2	3400	710	.21	480	.14*
WBT	3	5100	1700	.33*	1090	.21
WBR	d	1700	260	.15	160	.09
Right Turn Adjustment				NBR	.14*	
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.57		.78	

4. Glenn Ranch & Portola

Existing Counts						2011 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	1	1700	83	.05	.05		NBL	1	1700	.88	.05	.06
NBT	2	3400	18	.01*	.02*		NBT	2	3400	.19	.01*	.02*
NBR	0	0	32	.02	.04		NBR	0	0	.34	.02	.04
SBL	2	3400	349	.10*	.10*		SBL	2	3400	.370	.11*	.10*
SBT	2	3400	54	.02	.01		SBT	2	3400	.57	.02	.01
SBR	f		471		283		SBR			499		300
EBL	2	3400	343	.10*	.21		EBL	2	3400	.364	.11*	.22*
EBT	3	5100	619	.12	.1837		EBT	3	5100	.656	.13	.38
EBR	1	1700	77	.05	.03		EBR	1	1700	.82	.05	.03
WBL	2	3400	174	.05	.01*		WBL	2	3400	.184	.05	.01
WBT	3	5100	1715	.34*	.16		WBT	3	5100	.1818	.36*	.17*
WBR	1	1700	300	.18	.13		WBR	1	1700	.318	.19	.14
Right Turn Adjustment				NBR	.01*		Right Turn Adjustment			NBR	.01*	
Clearance Interval			.05*		.05*		Clearance Interval			.05*	.05*	
TOTAL CAPACITY UTILIZATION			.60		.55	TOTAL CAPACITY UTILIZATION			.64		.57	

2011 With-Project						2015 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	1	1700	92	.05	.06		NBL	1	1700	.60	.04	.05
NBT	2	3400	17	.01*	.02*		NBT	2	3400	.20	.01*	.02*
NBR	0	0	31	.02	.04		NBR	0	0	.30	.02	.04
SBL	2	3400	378	.11*	.11*		SBL	2	3400	.510	.15*	.10*
SBT	2	3400	55	.02	.01		SBT	2	3400	.50	.01	.01
SBR	f		485		285		SBR			940		670
EBL	2	3400	376	.11*	.22*		EBL	2	3400	.460	.14*	.31*
EBT	3	5100	528	.10	1613		EBT	3	5100	.760	.15	2080
EBR	1	1700	86	.05	.02		EBR	1	1700	.30	.02	.04
WBL	2	3400	196	.06	.02		WBL	2	3400	.110	.03	.01
WBT	3	5100	1390	.27*	.14*		WBT	3	5100	.1870	.37*	.21*
WBR	1	1700	306	.18	.13		WBR	1	1700	.60	.04	.22
Clearance Interval			.05*		.05*		Clearance Interval			.05*	.05*	
TOTAL CAPACITY UTILIZATION			.55		.54	TOTAL CAPACITY UTILIZATION			.72		.69	
Note: Assumes Right-Turn Overlap for WBR												

4. Glenn Ranch & Portola

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	60	.04	80	.05
NBT	2	3400	20	.01*	20	.01*
NBR	0	0	30	.02	70	.04
SBL	2	3400	510	.15*	360	.11*
SBT	2	3400	50	.01	20	.01
SBR	f		910		670	
EBL	2	3400	440	.13*	1060	.31*
EBT	3	5100	570	.11	1670	.33
EBR	1	1700	30	.02	70	.04
WBL	2	3400	110	.03	50	.01
WBT	3	5100	1470	.29*	840	.16*
WBR	1	1700	60	.04	390	.23
Clearance Interval			.05*		.05*	
Note: Assumes Right-Turn Overlap for WBR						
TOTAL CAPACITY UTILIZATION			.63		.64	

5. Portola & SR-241 Ramps

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	515	.15*	162	.05*	NBL	2	3400	546	.16*	172	.05*
NBT	3	5100	1133	.22	726	.14	NBT	3	5100	1201	.24	770	.15
NBR	f		56		59		NBR	f		59		63	
SBL	2	3400	193	.06	688	.20	SBL	2	3400	205	.06	729	.21
SBT	2	3400	557	.16*	1454	.43*	SBT	2	3400	590	.17*	1541	.45*
SBR	f		248		159		SBR	f		263		169	
EBL	1	1700	136	.08*	162	.10*	EBL	1	1700	144	.08*	172	.10*
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	f		115		354		EBR	f		122		375	
WBL	2	3400	93	.03	56	.02	WBL	2	3400	99	.03	59	.02
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	f		868		270		WBR	f		920		286	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.44		.63		TOTAL CAPACITY UTILIZATION			.46		.65	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	508	.15*	185	.05*	NBL	2	3400	630	.19*	300	.09*
NBT	3	5100	889	.17	608	.12	NBT	3	5100	1300	.25	1150	.23
NBR	f		87		116		NBR	f		10		20	
SBL	2	3400	155	.05	727	.21	SBL	2	3400	270	.08	960	.28
SBT	2	3400	516	.15*	1214	.36*	SBT	2	3400	820	.24*	1490	.44*
SBR	f		263		171		SBR	f		270		90	
EBL	1	1700	141	.08*	169	.10*	EBL	1	1700	120	.07*	190	.11*
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	f		137		344		EBR	f		260		480	
WBL	2	3400	277	.08	71	.02	WBL	2	3400	50	.01	10	.00
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	f		809		270		WBR	f		1610		380	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.43		.56		TOTAL CAPACITY UTILIZATION			.55		.69	

5. Portola & SR-241 Ramps

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	550	.16*	280	.08
NBT	3	5100	940	.18	950	.19*
NBR	f		40		100	
SBL	2	3400	210	.06	970	.29*
SBT	2	3400	690	.20*	1110	.33
SBR	f		260		90	
EBL	1	1700	120	.07*	180	.11*
EBT	0	0	0		0	
EBR	f		240		420	
WBL	2	3400	130	.04	30	.01
WBT	0	0	0		0	
WBR	f		1540		370	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.48		.64	

7. Lake Forest & SR-241 NB

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	81	.02*	223	.07	NBL	2	3400	86	.03*	236	.07
NBT	2	3400	805	.24	1126	.33*	NBT	2	3400	853	.25	1194	.35*
NBR	0	0	0		0		NBR	0	0	0		0	
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	2	3400	818	.24*	776	.23	SBT	2	3400	867	.26*	823	.24
SBR	1	1700	89	.05	201	.12	SBR	1	1700	94	.06	213	.13
EBL	0	0	0		0		EBL	0	0	0		0	
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	0	0	0		0		EBR	0	0	0		0	
WBL	0	0	0		0		WBL	0	0	0		0	
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	0	0	0		0		WBR	0	0	0		0	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.31		.38		TOTAL CAPACITY UTILIZATION			.34		.40	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	89	.03	237	.07*	NBL	2	3400	120	.04*	270	.08
NBT	2	3400	831	.24*	988	.29	NBT	2	3400	970	.29	1330	.39*
NBR	0	0	0		0		NBR	0	0	0		0	
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	2	3400	559	.16	780	.23*	SBT	2	3400	970	.29*	890	.26
SBR	1	1700	119	.07	211	.12	SBR	1	1700	80	.05	320	.19
EBL	0	0	0		0		EBL	0	0	0		0	
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	0	0	0		0		EBR	0	0	0		0	
WBL	0	0	0		0		WBL	0	0	0		0	
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	0	0	0		0		WBR	0	0	0		0	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.29		.35		TOTAL CAPACITY UTILIZATION			.38		.44	

7. Lake Forest & SR-241 NB

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	140	.04	290	.09*
NBT	2	3400	890	.26*	1040	.31
NBR	0	0	0		0	
SBL	0	0	0		0	
SBT	2	3400	720	.21	810	.24*
SBR	1	1700	90	.05	310	.18
EBL	0	0	0		0	
EBT	0	0	0		0	
EBR	0	0	0		0	
WBL	0	0	0		0	
WBT	0	0	0		0	
WBR	0	0	0		0	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.31		.38	

8. Lake Forest & SR-241 SB

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	2	3400	678	.20	1268	.37*	NBT	2	3400	719	.21	1344	.40*
NBR	0	0	0		0		NBR	0	0		0		
SBL	0	0	0		0		SBL	0	0		0		
SBT	2	3400	822	.24*	771	.23	SBT	2	3400	871	.26*	817	.24
SBR	0	0	0		0		SBR	0	0		0		
EBL	2	3400	215	.06*	88	.03*	EBL	2	3400	228	.07*	93	.03*
EBT	0	0	0		0		EBT	0	0		0		
EBR	1	1700	318	.19	124	.07	EBR	1	1700	337	.20	131	.08
WBL	0	0	0		0		WBL	0	0		0		
WBT	0	0	0		0		WBT	0	0		0		
WBR	0	0	0		0		WBR	0	0		0		
Right Turn Adjustment		EBR	.13*				Right Turn Adjustment	EBR	.13*				
Clearance Interval			.05*		.05*		Clearance Interval		.05*		.05*		
TOTAL CAPACITY UTILIZATION			.48		.45		TOTAL CAPACITY UTILIZATION			.51		.48	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	2	3400	702	.21*	1123	.33*	NBT	2	3400	790	.23	1480	.44*
NBR	0	0	0		0		NBR	0	0		0		
SBL	0	0	0		0		SBL	0	0		0		
SBT	2	3400	563	.17	774	.23	SBT	2	3400	970	.29*	890	.26
SBR	0	0	0		0		SBR	0	0		0		
EBL	2	3400	226	.07*	109	.03*	EBL	2	3400	290	.09*	120	.04*
EBT	0	0	0		0		EBT	0	0		0		
EBR	1	1700	320	.19	149	.09	EBR	1	1700	270	.16	190	.11
WBL	0	0	0		0		WBL	0	0		0		
WBT	0	0	0		0		WBT	0	0		0		
WBR	0	0	0		0		WBR	0	0		0		
Right Turn Adjustment		EBR	.09*				Right Turn Adjustment	EBR	.07*				
Clearance Interval			.05*		.05*		Clearance Interval		.05*		.05*		
TOTAL CAPACITY UTILIZATION			.42		.41		TOTAL CAPACITY UTILIZATION			.50		.53	

8. Lake Forest & SR-241 SB

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	0	0	0		0	
NBT	2	3400	730	.21	1210	.36*
NBR	0	0	0		0	
SBL	0	0	0		0	
SBT	2	3400	720	.21*	810	.24
SBR	0	0	0		0	
EBL	2	3400	290	.09*	120	.04*
EBT	0	0	0		0	
EBR	1	1700	280	.16	200	.12
WBL	0	0	0		0	
WBT	0	0	0		0	
WBR	0	0	0		0	
Right Turn Adjustment		EBC	.07*			
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.42		.45	

9. Bake & Rancho N

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	2	3400	1635	.48*	1067	.31	NBT	2	3400	1733	.51*	1131	.33
NBR	d	1700	219	.13	53	.03	NBR	d	1700	232	.14	56	.03
SBL	1	1700	265	.16*	49	.03	SBL	1	1700	281	.17*	52	.03
SBT	2	3400	745	.22	1886	.55*	SBT	2	3400	790	.23	1999	.59*
SBR	0	0	0		0		SBR	0	0		0		
EBL	0	0	0		0		EBL	0	0		0		
EBT	0	0	0		0		EBT	0	0		0		
EBR	0	0	0		0		EBR	0	0		0		
WBL	2	3400	35	.01*	199	.06*	WBL	2	3400	37	.01*	211	.06*
WBT	0	0	0		0		WBT	0	0		0		
WBR	2	3400	26	.01	158	.05	WBR	2	3400	28	.01	167	.05
Clearance Interval			.05*		.05*		Clearance Interval			.05*			
TOTAL CAPACITY UTILIZATION			.70		.66		TOTAL CAPACITY UTILIZATION			.74			
2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	2	3400	1662	.49*	973	.29	NBT	2	3400	700	.21	1770	.52*
NBR	d	1700	329	.19	354	.21	NBR	d	1700	190	.11	290	.17
SBL	1	1700	278	.16*	11	.01	SBL	1	1700	70	.04	150	.09*
SBT	2	3400	673	.20	1908	.56*	SBT	2	3400	1660	.49*	870	.26
SBR	0	0	0		0		SBR	0	0		0		
EBL	0	0	0		0		EBL	0	0		0		
EBT	0	0	0		0		EBT	0	0		0		
EBR	0	0	0		0		EBR	0	0		0		
WBL	2	3400	344	.10*	317	.09*	WBL	2	3400	150	.04*	240	.07*
WBT	0	0	0		0		WBT	0	0		0		
WBR	2	3400	3	.00	159	.05	WBR	2	3400	70	.02	200	.06
Clearance Interval			.05*		.05*		Clearance Interval			.05*			
TOTAL CAPACITY UTILIZATION			.80		.70		TOTAL CAPACITY UTILIZATION			.58			

9. Bake & Rancho N

2015 With-Project						
	LANES	CAPACITY	AM VOL	PK V/C	PM VOL	HOUR V/C
NBL	0	0	0		0	
NBT	2	3400	640	.19	1630	.48*
NBR	d	1700	290	.17	550	.32
SBL	1	1700	60	.04	150	.09*
SBT	2	3400	1470	.43*	770	.23
SBR	0	0	0		0	
EBL	0	0	0		0	
EBT	0	0	0		0	
EBR	0	0	0		0	
WBL	2	3400	610	.18*	400	.12*
WBT	0	0	0		0	
WBR	2	3400	30	.01	180	.05
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.66		.74	

10. Lake Forest & Rancho

Existing Counts						2011 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	102	.06*	.10		NBL	1	1700	.06*	.11*
NBT	2	3400	547	.16	.26*		NBT	2	3400	.17	.27
NBR	d	1700	90	.05	.01		NBR	d	1700	.06	.01
SBL	1	1700	159	.09	.05*		SBL	1	1700	.10	.05
SBT	2	3400	852	.25*	.20		SBT	2	3400	.27*	.22*
SBR	d	1700	92	.05	.05		SBR	d	1700	.06	.05
EBL	1	1700	40	.02	.08*		EBL	1	1700	.02	.08*
EBT	1	1700	62	.04*	.01		EBT	1	1700	.04*	.01
EBR	1	1700	47	.03	.09		EBR	1	1700	.03	.10
WBL	1	1700	6	.00	.04		WBL	1	1700	.00	.04
WBT	2	3400	10	.00	.03*		WBT	2	3400	.00	.03*
WBR	1	1700	4	.00	.07		WBR	1	1700	.00	.08
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.40		.47	TOTAL CAPACITY UTILIZATION			.42		.49

2011 With-Project						2015 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	10	.01	.08*		NBL	1	1700	.13*	.15
NBT	2	3400	606	.18*	.26		NBT	2	3400	.20	.34*
NBR	d	1700	160	.09	.16		NBR	d	1700	.13	.07
SBL	1	1700	81	.05*	.04		SBL	1	1700	.15	.07*
SBT	2	3400	758	.22	.24*		SBT	2	3400	.25*	.25
SBR	d	1700	7	.00	.01		SBR	d	1700	.14	.08
EBL	1	1700	6	.00	.02		EBL	1	1700	.03	.10
EBT	1	1700	199	.12*	.30*		EBT	1	1700	.03*	.05*
EBR	1	1700	35	.02	.03		EBR	1	1700	.05	.10
WBL	1	1700	367	.22*	.10*		WBL	1	1700	.04*	.13*
WBT	2	3400	474	.14	.10		WBT	2	3400	.01	.03
WBR	1	1700	1	.00	.03		WBR	1	1700	.04	.16
Clearance Interval			.05*		.05*	Right Turn Adjustment				WBR	.03*
TOTAL CAPACITY UTILIZATION			.62		.77	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.50		.67						

10. Lake Forest & Rancho

2015 With-Project						2015 With-Project & Improvements							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	120	.07	240	.14*	NBL	1	1700	120	.07	240	.14*
NBT	2	3400	680	.20*	1090	.32	NBT	2	3400	680	.20*	1090	.32
NBR	d	1700	330	.19	460	.27	NBR	d	1700	330	.19	460	.27
SBL	1	1700	190	.11*	120	.07	SBL	1	1700	190	.11*	120	.07
SBT	2	3400	810	.24	850	.25*	SBT	2	3400	810	.24	850	.25*
SBR	d	1700	60	.04	70	.04	SBR	d	1700	60	.04	70	.04
EBL	1	1700	20	.01	30	.02	EBL	1	1700	20	.01	30	.02
EBT	1	1700	170	.10*	490	.29*	EBT	2	3400	170	.05*	490	.14*
EBR	1	1700	60	.04	100	.06	EBR	1	1700	60	.04	100	.06
WBL	1	1700	260	.15*	330	.19*	WBL	1	1700	260	.15*	330	.19*
WBT	2	3400	660	.19	330	.10	WBT	2	3400	660	.19	330	.10
WBR	1	1700	60	.04	210	.12	WBR	1	1700	60	.04	210	.12
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.61		.92		TOTAL CAPACITY UTILIZATION			.56		.77	

11. Bake & Rancho S

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	79	.05	170	.10*	NBL	1	1700	84	.05	180	.11*
NBT	2	3400	1696	.50*	976	.29	NBT	2	3400	1798	.53*	1035	.30
NBR	0	0	0		0		NBR	0	0	0		0	
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	2	3400	699	.21	1838	.54*	SBT	2	3400	741	.22	1948	.57*
SBR	1	1700	94	.06	251	.15	SBR	1	1700	100	.06	266	.16
EBL	2	3400	167	.05*	159	.05*	EBL	2	3400	177	.05*	169	.05*
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	1	1700	50	.03	177	.10	EBR	1	1700	53	.03	188	.11
WBL	0	0	0		0		WBL	0	0	0		0	
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	0	0	0		0		WBR	0	0	0		0	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.60		.74		TOTAL CAPACITY UTILIZATION			.63		.78	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	62	.04	195	.11*	NBL	1	1700	130	.08*	60	.04
NBT	2	3400	1821	.54*	1125	.33	NBT	2	3400	780	.23	1870	.55*
NBR	0	0	0		0		NBR	0	0	0		0	
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	2	3400	892	.26	1967	.58*	SBT	2	3400	1540	.45*	970	.29
SBR	1	1700	138	.08	263	.15	SBR	1	1700	200	.12	230	.14
EBL	2	3400	181	.05*	219	.06*	EBL	2	3400	150	.04*	210	.06*
EBT	0	0	0		0		EBT	0	0	0		0	
EBR	1	1700	51	.03	168	.10	EBR	1	1700	30	.02	130	.08
WBL	0	0	0		0		WBL	0	0	0		0	
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	0	0	0		0		WBR	0	0	0		0	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.64		.80		TOTAL CAPACITY UTILIZATION			.62		.66	

11. Bake & Rancho S

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	110	.06*	50	.03
NBT	2	3400	810	.24	1910	.56*
NBR	0	0	0		0	
SBL	0	0	0		0	
SBT	2	3400	1600	.47*	1010	.30
SBR	1	1700	410	.24	250	.15
EBL	2	3400	170	.05*	290	.09*
EBT	0	0	0		0	
EBR	1	1700	30	.02	120	.07
WBL	0	0	0		0	
WBT	0	0	0		0	
WBR	0	0	0		0	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.63		.70	

12. El Toro & Portola/Santa M

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	308	.18*	264	.16*	NBL	1	1700	326	.19*	280	.16*
NBT	3	5100	171	.03	521	.10	NBT	3	5100	181	.04	552	.11
NBR	f		387		611		NBR	f		410		648	
SBL	1	1700	31	.02	26	.02	SBL	1	1700	33	.02	28	.02
SBT	3	5100	618	.12*	243	.05*	SBT	3	5100	655	.13*	258	.05*
SBR	1	1700	231	.14	88	.05	SBR	1	1700	245	.14	93	.05
EBL	2	3400	44	.01	176	.05	EBL	2	3400	47	.01	187	.06
EBT	3	5100	451	.09*	1304	.26*	EBT	3	5100	478	.09*	1382	.27*
EBR	1	1700	267	.16	376	.22	EBR	1	1700	283	.17	399	.23
WBL	2	3400	634	.19*	459	.14*	WBL	2	3400	672	.20*	487	.14*
WBT	4	6800	1122	.17	621	.09	WBT	4	6800	1189	.17	658	.10
WBR	d	1700	24	.01	58	.03	WBR	d	1700	25	.01	61	.04
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.63		.66		TOTAL CAPACITY UTILIZATION			.66		.67	
2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	430	.25*	336	.20*	NBL	1	1700	350	.21*	360	.21*
NBT	3	5100	155	.03	514	.10	NBT	3	5100	150	.03	480	.09
NBR	f		362		496		NBR	f		350		600	
SBL	1	1700	35	.02	28	.02	SBL	1	1700	50	.03	330	.19
SBT	3	5100	590	.12*	197	.04*	SBT	3	5100	500	.10*	590	.12*
SBR	1	1700	420	.25	193	.11	SBR	1	1700	170	.10	610	.36
EBL	2	3400	68	.02	355	.10	EBL	2	3400	40	.01	330	.10
EBT	3	5100	618	.12*	1679	.33*	EBT	3	5100	590	.12*	1130	.22*
EBR	1	1700	313	.18	478	.28	EBR	1	1700	330	.19	650	.38
WBL	2	3400	532	.16*	464	.14*	WBL	2	3400	690	.20*	470	.14*
WBT	4	6800	1471	.22	784	.12	WBT	4	6800	1380	.20	900	.13
WBR	d	1700	27	.02	54	.03	WBR	d	1700	20	.01	50	.03
Right Turn Adjustment		SBR	.08*				Right Turn Adjustment			SBR	.07*		
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.78		.76		TOTAL CAPACITY UTILIZATION			.68		.81	

12. El Toro & Portola/Santa M

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	370	.22*	400	.24*
NBT	3	5100	160	.03	420	.08
NBR	f		310		470	
SBL	1	1700	50	.03	340	.20
SBT	3	5100	470	.09*	560	.11*
SBR	1	1700	300	.18	660	.39
EBL	2	3400	50	.01*	430	.13
EBT	3	5100	620	.12	1380	.27*
EBR	1	1700	390	.23	690	.41
WBL	2	3400	450	.13	430	.13*
WBT	4	6800	1700	.25*	1020	.15
WBR	d	1700	20	.01	40	.02
Right Turn Adjustment		SBR	.08*		SBR	.09*
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.70		.89	

15. Lake Forest & Trabuco

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	178	.05*	256	.08*	NBL	2	3400	189	.06*	271	.08*
NBT	3	5100	862	.17	976	.19	NBT	3	5100	914	.18	1035	.20
NBR	1	1700	100	.06	423	.25	NBR	1	1700	106	.06	448	.26
SBL	2	3400	165	.05	341	.10	SBL	2	3400	175	.05	361	.11
SBT	3	5100	1227	.28*	1062	.25*	SBT	3	5100	1301	.30*	1126	.26*
SBR	0	0	221		194		SBR	0	0	234		206	
EBL	2	3400	142	.04*	320	.09	EBL	2	3400	151	.04*	339	.10
EBT	3	5100	431	.08	1115	.22*	EBT	3	5100	457	.09	1182	.23*
EBR	1	1700	305	.18	146	.09	EBR	1	1700	323	.19	155	.09
WBL	2	3400	372	.11	177	.05*	WBL	2	3400	394	.12	188	.06*
WBT	3	5100	1095	.21*	568	.11	WBT	3	5100	1161	.23*	602	.12
WBR	1	1700	377	.22	272	.16	WBR	1	1700	400	.24	288	.17
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.63		.65		TOTAL CAPACITY UTILIZATION			.68		.68	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	198	.06*	266	.08*	NBL	2	3400	260	.08*	270	.08
NBT	3	5100	957	.19	1074	.21	NBT	3	5100	870	.17	1020	.20*
NBR	1	1700	79	.05	496	.29	NBR	1	1700	90	.05	690	.41
SBL	2	3400	142	.04	220	.06	SBL	2	3400	330	.10	500	.15*
SBT	3	5100	1346	.31*	1201	.28*	SBT	3	5100	1130	.25*	1010	.23
SBR	0	0	239		210		SBR	0	0	170		140	
EBL	2	3400	160	.05*	329	.10	EBL	2	3400	150	.04	220	.06
EBT	3	5100	455	.09	1190	.23*	EBT	3	5100	630	.12*	1170	.23*
EBR	1	1700	313	.18	146	.09	EBR	1	1700	440	.26	200	.12
WBL	2	3400	398	.12	183	.05*	WBL	2	3400	690	.20*	320	.09*
WBT	3	5100	1186	.23*	615	.12	WBT	3	5100	1080	.21	590	.12
WBR	1	1700	252	.15	229	.13	WBR	1	1700	550	.32	520	.31
Clearance Interval			.05*		.05*		Right Turn Adjustment			EBR	.08*	NBR	.14*
TOTAL CAPACITY UTILIZATION			.70		.69		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.78		.86								

15. Lake Forest & Trabuco

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	260	.08*	270	.08*
NBT	3	5100	880	.17	1070	.21
NBR	1	1700	80	.05	740	.44
SBL	2	3400	270	.08	340	.10
SBT	3	5100	1170	.27*	1090	.25*
SBR	0	0	200		160	
EBL	2	3400	170	.05	220	.06
EBT	3	5100	610	.12*	1160	.23*
EBR	1	1700	440	.26	210	.12
WBL	2	3400	720	.21*	280	.08*
WBT	3	5100	1040	.20	580	.11
WBR	1	1700	390	.23	440	.26
Right Turn Adjustment		EBC	.08*		NBR	.15*
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.81		.84	

16. Ridge Route & Trabuco

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	231	.14*	167	.10*	NBL	1	1700	245	.14*	177	.10*
NBT	0	0	0		0		NBT	0	0	0		0	
NBR	1	1700	218	.13	295	.17	NBR	1	1700	231	.14	313	.18
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	0	0	0		0		SBT	0	0	0		0	
SBR	0	0	0		0		SBR	0	0	0		0	
EBL	0	0	0		0		EBL	0	0	0		0	
EBT	3	5100	543	.11	1724	.34*	EBT	3	5100	576	.11	1827	.36*
EBR	d	1700	286	.17	167	.10	EBR	d	1700	303	.18	177	.10
WBL	1	1700	234	.14	138	.08*	WBL	1	1700	248	.15	146	.09*
WBT	3	5100	1548	.30*	776	.15	WBT	3	5100	1641	.32*	823	.16
WBR	0	0	0		0		WBR	0	0	0		0	
Right Turn Adjustment					NBR	.01*	Right Turn Adjustment			NBR	.01*		
Clearance Interval			.05*			.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.49		.58		TOTAL CAPACITY UTILIZATION			.51		.61	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	252	.15*	182	.11*	NBL	1	1700	190	.11*	210	.12*
NBT	0	0	0		0		NBT	0	0	0		0	
NBR	1	1700	231	.14	310	.18	NBR	1	1700	100	.06	210	.12
SBL	0	0	0		0		SBL	0	0	0		0	
SBT	0	0	0		0		SBT	0	0	0		0	
SBR	0	0	0		0		SBR	0	0	0		0	
EBL	0	0	0		0		EBL	0	0	0		0	
EBT	3	5100	525	.10	1768	.35*	EBT	3	5100	720	.14	2050	.40*
EBR	d	1700	298	.18	149	.09	EBR	d	1700	200	.12	130	.08
WBL	1	1700	272	.16	143	.08*	WBL	1	1700	110	.06	100	.06*
WBT	3	5100	1523	.30*	775	.15	WBT	3	5100	1860	.36*	1110	.22
WBR	0	0	0		0		WBR	0	0	0		0	
Right Turn Adjustment					NBR	.01*	Right Turn Adjustment			.05*		.05*	
Clearance Interval			.05*			.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.50		.60		TOTAL CAPACITY UTILIZATION			.52		.63	

16. Ridge Route & Trabuco

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	190	.11*	210	.12*
NBT	0	0	0		0	
NBR	1	1700	90	.05	200	.12
SBL	0	0	0		0	
SBT	0	0	0		0	
SBR	0	0	0		0	
EBL	0	0	0		0	
EBT	3	5100	680	.13	1950	.38*
EBR	d	1700	150	.09	100	.06
WBL	1	1700	120	.07	100	.06*
WBT	3	5100	1700	.33*	980	.19
WBR	0	0	0		0	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.49		.61	

17. El Toro & Trabuco

Existing Counts						2011 No-Project								
	LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C		LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C	
NBL	2	3400	257	.08*	327	.10		NBL	2	3400	272	.08*	347	.10
NBT	3	5100	689	.14	1169	.23*		NBT	3	5100	730	.14	1239	.24*
NBR	1	1700	123	.07	372	.22		NBR	1	1700	130	.08	394	.23
SBL	2	3400	197	.06	261	.08*		SBL	2	3400	209	.06	277	.08*
SBT	3	5100	1579	.31*	969	.19		SBT	3	5100	1674	.33*	1027	.20
SBR	1	1700	444	.26	190	.11		SBR	1	1700	471	.28	201	.12
EBL	2	3400	182	.05*	497	.15		EBL	2	3400	193	.06	527	.16
EBT	3	5100	306	.06	1119	.22*		EBT	3	5100	324	.06*	1186	.23*
EBR	d	1700	162	.10	202	.12		EBR	d	1700	172	.10	214	.13
WBL	2	3400	269	.08	144	.04*		WBL	2	3400	285	.08	153	.05
WBT	3	5100	977	.19*	459	.09		WBT	3	5100	1036	.20*	487	.10*
WBR	d	1700	166	.10	165	.10		WBR	d	1700	176	.10	175	.10
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*		
TOTAL CAPACITY UTILIZATION			.68		.62		TOTAL CAPACITY UTILIZATION			.72		.70		
Note: Assumes E/W Split Phasing														

2011 With-Project						2015 No-Project								
	LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C		LANES	CAPACITY	AM PK HOUR VOL	V/C	PM PK HOUR VOL	V/C	
NBL	2	3400	212	.06*	330	.10		NBL	2	3400	410	.12*	480	.14
NBT	3	5100	800	.16	1213	.24*		NBT	3	5100	1040	.20	1440	.28*
NBR	1	1700	131	.08	386	.23		NBR	1	1700	130	.08	570	.34
SBL	2	3400	216	.06	294	.09*		SBL	2	3400	290	.09	260	.08*
SBT	3	5100	1624	.32*	1040	.20		SBT	3	5100	1440	.28*	880	.17
SBR	1	1700	483	.28	208	.12		SBR	1	1700	450	.26	180	.11
EBL	2	3400	184	.05	505	.15		EBL	2	3400	200	.06*	590	.17
EBT	3	5100	290	.06*	1147	.22*		EBT	3	5100	320	.06	1160	.23*
EBR	d	1700	164	.10	215	.13		EBR	d	1700	260	.15	360	.21
WBL	2	3400	281	.08	148	.04		WBL	2	3400	280	.08	190	.06*
WBT	3	5100	1000	.20*	458	.09*		WBT	3	5100	1000	.20*	520	.10
WBR	d	1700	162	.10	183	.11		WBR	d	1700	220	.13	150	.09
Clearance Interval			.05*		.05*		Right Turn Adjustment			NBR		.01*		
Note: Assumes E/W Split Phasing												.05*		
TOTAL CAPACITY UTILIZATION			.69		.69		TOTAL CAPACITY UTILIZATION			.71		.71		

17. El Toro & Trabuco

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	340	.10*	450	.13
NBT	3	5100	1110	.22	1390	.27*
NBR	1	1700	120	.07	550	.32
SBL	2	3400	290	.09	270	.08*
SBT	3	5100	1400	.27*	930	.18
SBR	1	1700	410	.24	130	.08
EBL	2	3400	160	.05*	570	.17
EBT	3	5100	310	.06	1110	.22*
EBR	d	1700	270	.16	330	.19
WBL	2	3400	280	.08	190	.06*
WBT	3	5100	950	.19*	490	.10
WBR	d	1700	230	.14	160	.09
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.66		.68	

19. Lake Forest & Toledo

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	81	.05*	.04		NBL	1	1700	86	.05*	.04	
NBT	3	5100	1069	.21	1327	.26*	NBT	3	5100	1133	.22	1407	.28*
NBR	d	1700	69	.04	97	.06	NBR	d	1700	73	.04	103	.06
SBL	1	1700	66	.04	61	.04*	SBL	1	1700	70	.04	65	.04*
SBT	3	5100	1514	.30*	1222	.24	SBT	3	5100	1605	.31*	1295	.25
SBR	d	1700	64	.04	22	.01	SBR	d	1700	68	.04	23	.01
EBL	1	1700	27	.02	117	.07	EBL	1	1700	29	.02	124	.07
EBT	2	3400	86	.05*	337	.11*	EBT	2	3400	91	.05*	357	.12*
EBR	0	0	90	.05	48		EBR	0	0	95	.06	51	
WBL	1	1700	147	.09*	54	.03*	WBL	1	1700	156	.09*	57	.03*
WBT	2	3400	311	.11	73	.03	WBT	2	3400	330	.11	77	.03
WBR	0	0	48		25		WBR	0	0	51		26	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.54		.49		TOTAL CAPACITY UTILIZATION			.55		.52	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	92	.05*	60	.04	NBL	1	1700	70	.04*	60	.04
NBT	3	5100	1152	.23	1429	.28*	NBT	3	5100	800	.16	1420	.28*
NBR	d	1700	76	.04	97	.06	NBR	d	1700	30	.02	60	.04
SBL	1	1700	71	.04	70	.04*	SBL	1	1700	50	.03	60	.04*
SBT	3	5100	1623	.32*	1355	.27	SBT	3	5100	1650	.32*	1120	.22
SBR	d	1700	43	.03	21	.01	SBR	d	1700	30	.02	80	.05
EBL	1	1700	34	.02*	134	.08	EBL	1	1700	20	.01	80	.05
EBT	2	3400	90	.05	340	.11*	EBT	2	3400	90	.05*	190	.08*
EBR	0	0	94	.06	50		EBR	0	0	80		80	
WBL	1	1700	109	.06	35	.02*	WBL	1	1700	70	.04*	30	.02*
WBT	2	3400	329	.11*	72	.03	WBT	2	3400	170	.06	70	.04
WBR	0	0	54		27		WBR	0	0	30		50	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.55		.50		TOTAL CAPACITY UTILIZATION			.50		.47	

19. Lake Forest & Toledo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	70	.04*	50	.03
NBT	3	5100	810	.16	1440	.28*
NBR	d	1700	30	.02	40	.02
SBL	1	1700	50	.03	50	.03*
SBT	3	5100	1620	.32*	1140	.22
SBR	d	1700	40	.02	80	.05
EBL	1	1700	20	.01	110	.06*
EBT	2	3400	100	.05*	150	.07
EBR	0	0	80		80	
WBL	1	1700	60	.04*	30	.02
WBT	2	3400	140	.05	70	.04*
WBR	0	0	30		50	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.50		.46	

20. Ridge Route & Toledo

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	49	.03	20	.01	NBL	1	1700	52	.03*	21	.01
NBT	2	3400	305	.12*	319	.10*	NBT	2	3400	323	.13	338	.11*
NBR	0	0	109		22		NBR	0	0	116		23	
SBL	1	1700	64	.04*	18	.01*	SBL	1	1700	68	.04	19	.01*
SBT	2	3400	379	.13	243	.08	SBT	2	3400	402	.14*	258	.08
SBR	0	0	73		27		SBR	0	0	77		29	
EBL	1	1700	62	.04*	77	.05	EBL	1	1700	66	.04*	82	.05
EBT	2	3400	144	.05	345	.11*	EBT	2	3400	153	.06	366	.12*
EBR	0	0	40		36		EBR	0	0	42		38	
WBL	1	1700	115	.07	25	.01*	WBL	1	1700	122	.07	26	.02*
WBT	2	3400	279	.10*	109	.04	WBT	2	3400	296	.11*	116	.05
WBR	0	0	59		35		WBR	0	0	63		37	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.35		.28		TOTAL CAPACITY UTILIZATION			.37		.31	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	54	.03*	18	.01	NBL	1	1700	40	.02	40	.02
NBT	2	3400	343	.13	338	.11*	NBT	2	3400	220	.09*	330	.11*
NBR	0	0	114		19		NBR	0	0	80		40	
SBL	1	1700	59	.03	14	.01*	SBL	1	1700	70	.04*	60	.04*
SBT	2	3400	410	.14*	237	.08	SBT	2	3400	310	.11	190	.06
SBR	0	0	81		35		SBR	0	0	50		10	
EBL	1	1700	66	.04	82	.05	EBL	1	1700	50	.03	90	.05
EBT	2	3400	157	.06*	351	.11*	EBT	2	3400	140	.05*	310	.11*
EBR	0	0	41		36		EBR	0	0	30		50	
WBL	1	1700	136	.08*	31	.02*	WBL	1	1700	160	.09*	50	.03*
WBT	2	3400	242	.09	87	.04	WBT	2	3400	180	.07	70	.04
WBR	0	0	64		39		WBR	0	0	60		80	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.36		.30		TOTAL CAPACITY UTILIZATION			.32		.34	

20. Ridge Route & Toledo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	40	.02	40	.02
NBT	2	3400	210	.09*	340	.11*
NBR	0	0	90		40	
SBL	1	1700	60	.04*	50	.03*
SBT	2	3400	310	.11	190	.06
SBR	0	0	50		10	
EBL	1	1700	50	.03	90	.05
EBT	2	3400	150	.05*	260	.09*
EBR	0	0	30		40	
WBL	1	1700	140	.08*	60	.04*
WBT	2	3400	150	.06	60	.04
WBR	0	0	60		70	.04
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.31		.32	

21. El Toro & Toledo

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	131	.08*	80	.05	NBL	1	1700	139	.08*	85	.05
NBT	3	5100	1005	.20	1595	.31*	NBT	3	5100	1065	.21	1691	.33*
NBR	d	1700	11	.01	19	.01	NBR	d	1700	12	.01	20	.01
SBL	1	1700	2	.00	3	.00	SBL	1	1700	2	.00	3	.00
SBT	3	5100	1854	.36*	1317	.26	SBT	3	5100	1965	.39*	1396	.27
SBR	d	1700	207	.12	54	.03	SBR	d	1700	219	.13	57	.03
EBL	1.5		59	{.02}* [*]	304	{.10}* [*]	EBL	1.5		63	{.02}* [*]	322	{.10}* [*]
EBT	0.5	3400	19	.02	22	.10	EBT	0.5	3400	20	.02	23	.10
EBR	1	1700	110	.06	148	.09	EBR	1	1700	117	.07	157	.09
WBL	0	0	20		11		WBL	0	0	21		12	
WBT	1	1700	27	.03*	3	.01*	WBT	1	1700	29	.03*	3	.01*
WBR	0	0	6		3		WBR	0	0	6		3	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.54		.47		TOTAL CAPACITY UTILIZATION			.57		.49	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	144	.08*	96	.06	NBL	1	1700	130	.08*	120	.07
NBT	3	5100	1076	.21	1671	.33*	NBT	3	5100	1560	.31	2180	.43*
NBR	d	1700	13	.01	20	.01	NBR	d	1700	10	.01	20	.01
SBL	1	1700	2	.00	3	.00	SBL	1	1700	10	.01	10	.01*
SBT	3	5100	2029	.40*	1399	.27	SBT	3	5100	1940	.38*	1290	.25
SBR	d	1700	178	.10	30	.02	SBR	d	1700	170	.10	70	.04
EBL	1.5		63	{.03}* [*]	313	{.10}* [*]	EBL	1.5		50	{.02}* [*]	240	{.08}* [*]
EBT	0.5	3400	23	.03	21	.10	EBT	0.5	3400	10	.02	20	.08
EBR	1	1700	107	.06	150	.09	EBR	1	1700	120	.07	140	.08
WBL	0	0	21		12		WBL	0	0	20		10	
WBT	1	1700	29	.03*	2	.01*	WBT	1	1700	20	.03*	10	.02*
WBR	0	0	6		3		WBR	0	0	10		10	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.59		.49		TOTAL CAPACITY UTILIZATION			.56		.59	

21. El Toro & Toledo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	140	.08*	110	.06
NBT	3	5100	1540	.30	2130	.42*
NBR	d	1700	10	.01	20	.01
SBL	1	1700	10	.01	10	.01*
SBT	3	5100	1980	.39*	1310	.26
SBR	d	1700	130	.08	70	.04
EBL	1.5		50	{.02}* [*]	200	{.06}* [*]
EBT	0.5	3400	10	.02	20	.06
EBR	1	1700	120	.07	140	.08
WBL	0	0	20		10	
WBT	1	1700	20	.03*	10	.02*
WBR	0	0	10		10	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.57		.56	

23. Lake Forest & Jeronimo

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	100	.06*	75	.04	NBL	1	1700	106	.06*	79	.05
NBT	3	5100	912	.18	1274	.25*	NBT	3	5100	967	.19	1350	.26*
NBR	1	1700	140	.08	193	.11	NBR	1	1700	148	.09	205	.12
SBL	1	1700	174	.10	143	.08*	SBL	1	1700	184	.11	152	.09*
SBT	3	5100	1226	.24*	1067	.21	SBT	3	5100	1300	.25*	1131	.22
SBR	1	1700	290	.17	94	.06	SBR	1	1700	307	.18	100	.06
EBL	1	1700	71	.04*	144	.08	EBL	1	1700	75	.04*	153	.09
EBT	2	3400	205	.09	643	.21*	EBT	2	3400	217	.10	682	.23*
EBR	0	0	113		80		EBR	0	0	120		85	
WBL	1	1700	228	.13	85	.05*	WBL	1	1700	242	.14	90	.05*
WBT	2	3400	604	.23*	152	.08	WBT	2	3400	640	.25*	161	.08
WBR	0	0	185		112		WBR	0	0	196		119	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.62		.64		TOTAL CAPACITY UTILIZATION			.65		.68	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	96	.06*	69	.04	NBL	1	1700	80	.05*	70	.04
NBT	3	5100	992	.19	1342	.26*	NBT	3	5100	780	.15	1610	.32*
NBR	1	1700	136	.08	208	.12	NBR	1	1700	130	.08	180	.11
SBL	1	1700	187	.11	159	.09*	SBL	1	1700	220	.13	120	.07*
SBT	3	5100	1255	.25*	1150	.23	SBT	3	5100	1340	.26*	1030	.20
SBR	1	1700	320	.19	111	.07	SBR	1	1700	210	.12	200	.12
EBL	1	1700	81	.05*	167	.10	EBL	1	1700	80	.05	150	.09
EBT	2	3400	213	.10	639	.21*	EBT	2	3400	310	.13*	670	.21*
EBR	0	0	122		78		EBR	0	0	120		60	
WBL	1	1700	256	.15	80	.05*	WBL	1	1700	340	.20*	160	.09*
WBT	2	3400	611	.24*	169	.09	WBT	2	3400	590	.25	270	.11
WBR	0	0	192		127		WBR	0	0	250		90	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.65		.66		TOTAL CAPACITY UTILIZATION			.69		.74	

23. Lake Forest & Jeronimo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	90	.05*	70	.04
NBT	3	5100	790	.15	1610	.32*
NBR	1	1700	120	.07	180	.11
SBL	1	1700	220	.13	120	.07*
SBT	3	5100	1330	.26*	1060	.21
SBR	1	1700	210	.12	200	.12
EBL	1	1700	80	.05	150	.09
EBT	2	3400	300	.12*	660	.21*
EBR	0	0	120		50	
WBL	1	1700	320	.19*	160	.09*
WBT	2	3400	560	.24	270	.11
WBR	0	0	250		90	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.67		.74	

24. Ridge Route & Jeronimo

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	65	.04	48	.03	NBL	1	1700	69	.04	51	.03
NBT	2	3400	243	.07*	323	.10*	NBT	2	3400	258	.08*	342	.10*
NBR	d	1700	80	.05	136	.08	NBR	d	1700	85	.05	144	.08
SBL	1	1700	160	.09*	67	.04*	SBL	1	1700	170	.10*	71	.04*
SBT	2	3400	320	.09	241	.07	SBT	2	3400	339	.10	255	.08
SBR	d	1700	178	.10	29	.02	SBR	d	1700	189	.11	31	.02
EBL	1	1700	112	.07*	59	.03	EBL	1	1700	119	.07*	63	.04
EBT	2	3400	303	.13	794	.25*	EBT	2	3400	321	.14	842	.27*
EBR	0	0	147		61		EBR	0	0	156		65	
WBL	1	1700	123	.07	77	.05*	WBL	1	1700	130	.08	82	.05*
WBT	2	3400	700	.24*	285	.11	WBT	2	3400	742	.25*	302	.11
WBR	0	0	110		72		WBR	0	0	117		76	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.52		.49		TOTAL CAPACITY UTILIZATION			.55		.51	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	63	.04	51	.03	NBL	1	1700	170	.10*	60	.04
NBT	2	3400	265	.08*	344	.10*	NBT	2	3400	270	.08	310	.09*
NBR	d	1700	87	.05	137	.08	NBR	d	1700	70	.04	150	.09
SBL	1	1700	170	.10*	72	.04*	SBL	1	1700	20	.01	100	.06*
SBT	2	3400	304	.09	254	.07	SBT	2	3400	230	.07*	190	.06
SBR	d	1700	237	.14	30	.02	SBR	d	1700	20	.01	40	.02
EBL	1	1700	120	.07*	68	.04	EBL	1	1700	130	.08*	70	.04
EBT	2	3400	316	.14	804	.26*	EBT	2	3400	640	.21	1010	.32*
EBR	0	0	160		69		EBR	0	0	90		70	
WBL	1	1700	163	.10	81	.05*	WBL	1	1700	10	.01	100	.06*
WBT	2	3400	691	.24*	308	.11	WBT	2	3400	460	.15*	360	.13
WBR	0	0	128		69		WBR	0	0	40		90	
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.54		.50		TOTAL CAPACITY UTILIZATION			.45		.58	

24. Ridge Route & Jeronimo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	160	.09*	60	.04
NBT	2	3400	280	.08	310	.09*
NBR	d	1700	70	.04	160	.09
SBL	1	1700	20	.01	100	.06*
SBT	2	3400	230	.07*	200	.06
SBR	d	1700	20	.01	40	.02
EBL	1	1700	130	.08*	70	.04
EBT	2	3400	630	.21	970	.31*
EBR	0	0	90		70	
WBL	1	1700	10	.01	90	.05*
WBT	2	3400	430	.14*	360	.13
WBR	0	0	40		90	
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.43		.56	

25. El Toro & Jeronimo

Existing Counts						2011 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	81	.05*	.05		NBL	1	1700	.05*	.05
NBT	3	5100	853	.17	.28*		NBT	3	5100	.18	.30*
NBR	1	1700	247	.15	.21		NBR	1	1700	.15	.22
SBL	1	1700	237	.14	.18*		SBL	1	1700	.15	.19*
SBT	3	5100	1594	.31*	.21		SBT	3	5100	.33*	.22
SBR	d	1700	130	.08	.02		SBR	d	1700	.08	.02
EBL	1	1700	95	.06	.06		EBL	1	1700	.06	.06
EBT	2	3400	251	.13*	.23*		EBT	2	3400	.13*	.24*
EBR	0	0	176		104		EBR	0	0	187	
WBL	2	3400	443	.13*	.08*		WBL	2	3400	.14*	.08*
WBT	2	3400	508	.15	.08		WBT	2	3400	.16	.08
WBR	1	1700	181	.11	.12		WBR	1	1700	.11	.13
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.67		.82	TOTAL CAPACITY UTILIZATION			.70		.86

2011 With-Project						2015 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	85	.05*	.05		NBL	1	1700	.05	.04
NBT	3	5100	914	.18	.30*		NBT	3	5100	.28*	.34*
NBR	1	1700	259	.15	.22		NBR	1	1700	.14	.08
SBL	1	1700	248	.15	.18*		SBL	1	1700	.18*	.11*
SBT	3	5100	1706	.33*	.22		SBT	3	5100	.33	.19
SBR	d	1700	168	.10	.01		SBR	d	1700	.12	.20
EBL	1	1700	107	.06	.04		EBL	1	1700	.07*	.14*
EBT	2	3400	264	.13*	.24*		EBT	2	3400	.10	.17
EBR	0	0	186		126		EBR	0	0	40	
WBL	2	3400	455	.13*	.07*		WBL	2	3400	.09	.07
WBT	2	3400	510	.15	.09		WBT	2	3400	.20*	.15*
WBR	1	1700	179	.11	.14		WBR	1	1700	.05	.21
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.69		.84	TOTAL CAPACITY UTILIZATION			.78		.79

25. El Toro & Jeronimo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	80	.05	70	.04
NBT	3	5100	1420	.28*	1690	.33*
NBR	1	1700	250	.15	130	.08
SBL	1	1700	350	.21*	200	.12*
SBT	3	5100	1700	.33	980	.19
SBR	d	1700	160	.09	340	.20
EBL	1	1700	120	.07*	240	.14*
EBT	2	3400	300	.10	450	.17
EBR	0	0	40		130	
WBL	2	3400	280	.08	230	.07
WBT	2	3400	680	.20*	510	.15*
WBR	1	1700	80	.05	340	.20
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.81		.79	

26. Los Alisos & Jeronimo

Existing Counts						2011 No-Project									
	LANES	CAPACITY	AM PK HOUR		V/C	PM PK HOUR		V/C	LANES	CAPACITY	AM PK HOUR		PM PK HOUR		
			VOL	V/C		VOL	V/C				VOL	V/C	VOL	V/C	
NBL	1	1700	161	.09*		202	.12		NBL	1	1700	171	.10*	214	.13
NBT	3	5100	542	.11		1351	.26*		NBT	3	5100	575	.11	1432	.28*
NBR	d	1700	63	.04		79	.05		NBR	d	1700	67	.04	84	.05
SBL	1	1700	267	.16		188	.11*		SBL	1	1700	283	.17	199	.12*
SBT	3	5100	1204	.24*		633	.12		SBT	3	5100	1276	.25*	671	.13
SBR	d	1700	156	.09		71	.04		SBR	d	1700	165	.10	75	.04
EBL	1	1700	150	.09*		388	.23*		EBL	1	1700	159	.09*	411	.24*
EBT	2	3400	516	.15		760	.22		EBT	2	3400	547	.16	806	.24
EBR	d	1700	166	.10		196	.12		EBR	d	1700	176	.10	208	.12
WBL	1	1700	196	.12		202	.12		WBL	1	1700	208	.12	214	.13
WBT	2	3400	625	.18*		471	.14*		WBT	2	3400	662	.19*	499	.15*
WBR	1	1700	126	.07		171	.10		WBR	1	1700	134	.08	181	.11
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*			
TOTAL CAPACITY UTILIZATION			.65		.79		TOTAL CAPACITY UTILIZATION			.68		.84			

2011 With-Project						2015 No-Project									
	LANES	CAPACITY	AM PK HOUR		V/C	PM PK HOUR		V/C	LANES	CAPACITY	AM PK HOUR		PM PK HOUR		
			VOL	V/C		VOL	V/C				VOL	V/C	VOL	V/C	
NBL	1	1700	183	.11*		222	.13		NBL	1	1700	160	.09*	210	.12
NBT	3	5100	550	.11		1428	.28*		NBT	3	5100	660	.13	1490	.29*
NBR	d	1700	81	.05		73	.04		NBR	d	1700	250	.15	310	.18
SBL	1	1700	289	.17		187	.11*		SBL	1	1700	320	.19	260	.15*
SBT	3	5100	1269	.25*		673	.13		SBT	3	5100	1280	.25*	970	.19
SBR	d	1700	138	.08		69	.04		SBR	d	1700	450	.26	130	.08
EBL	1	1700	161	.09*		395	.23		EBL	1	1700	170	.10	380	.22
EBT	2	3400	526	.15		806	.24*		EBT	2	3400	590	.17*	820	.24*
EBR	d	1700	188	.11		197	.12		EBR	d	1700	170	.10	200	.12
WBL	1	1700	211	.12		236	.14*		WBL	1	1700	340	.20*	260	.15*
WBT	2	3400	622	.18*		477	.14		WBT	2	3400	860	.25	390	.11
WBR	1	1700	140	.08		182	.11		WBR	1	1700	210	.12	250	.15
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*			
TOTAL CAPACITY UTILIZATION			.68		.82		TOTAL CAPACITY UTILIZATION			.76		.88			

26. Los Alisos & Jeronimo

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	160	.09*	200	.12
NBT	3	5100	680	.13	1460	.29*
NBR	d	1700	250	.15	330	.19
SBL	1	1700	290	.17	260	.15*
SBT	3	5100	1270	.25*	970	.19
SBR	d	1700	460	.27	140	.08
EBL	1	1700	170	.10	380	.22
EBT	2	3400	630	.19*	800	.24*
EBR	d	1700	190	.11	210	.12
WBL	1	1700	330	.19*	290	.17*
WBT	2	3400	850	.25	390	.11
WBR	1	1700	240	.14	250	.15
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.77		.90	

27. Lake Forest & Muirlands

Existing Counts						2011 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	2	3400	74	.02*	.02		NBL	2	3400	78	.02*	.02
NBT	3	5100	739	.14	.25*		NBT	3	5100	783	.15	.27*
NBR	1	1700	79	.05	.08		NBR	1	1700	84	.05	.08
SBL	2	3400	122	.04	.05*		SBL	2	3400	129	.04	.06*
SBT	3	5100	1188	.23*	.18		SBT	3	5100	1259	.25*	.19
SBR	1	1700	183	.11	.04		SBR	1	1700	194	.11	.04
EBL	2	3400	84	.02*	.02		EBL	2	3400	89	.03*	.02
EBT	2	3400	131	.04	.05*		EBT	2	3400	139	.04	.05*
EBR	1	1700	40	.02	.01		EBR	1	1700	42	.02	.01
WBL	2	3400	256	.08	.05*		WBL	2	3400	271	.08	.05*
WBT	2	3400	649	.19*	.06		WBT	2	3400	688	.20*	.06
WBR	1	1700	187	.11	.08		WBR	1	1700	198	.12	.09
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.51		.45	TOTAL CAPACITY UTILIZATION			.55		.48	

2011 With-Project						2015 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	2	3400	79	.02*	.02		NBL	2	3400	40	.01*	.02
NBT	3	5100	787	.15	.26*		NBT	3	5100	700	.14	.29*
NBR	1	1700	88	.05	.08		NBR	1	1700	110	.06	.17
SBL	2	3400	129	.04	.05*		SBL	2	3400	80	.02	.04*
SBT	3	5100	1293	.25*	.20		SBT	3	5100	1660	.33*	.20
SBR	1	1700	129	.08	.05		SBR	1	1700	220	.13	.06
EBL	2	3400	91	.03*	.01		EBL	2	3400	70	.02*	.10
EBT	2	3400	147	.04	.06*		EBT	2	3400	240	.07	.35*
EBR	1	1700	32	.02	.02		EBR	1	1700	30	.02	.11
WBL	2	3400	262	.08	.05*		WBL	2	3400	250	.07	.12*
WBT	2	3400	672	.20*	.06		WBT	2	3400	800	.24*	.07
WBR	1	1700	202	.12	.09		WBR	1	1700	110	.06	.05
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.55		.47	TOTAL CAPACITY UTILIZATION			.65		.85	

27. Lake Forest & Muirlands

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	40	.01*	70	.02
NBT	3	5100	710	.14	1520	.30*
NBR	1	1700	120	.07	300	.18
SBL	2	3400	70	.02	140	.04*
SBT	3	5100	1650	.32*	1020	.20
SBR	1	1700	190	.11	110	.06
EBL	2	3400	70	.02*	310	.09
EBT	2	3400	240	.07	1170	.34*
EBR	1	1700	30	.02	180	.11
WBL	2	3400	260	.08	430	.13*
WBT	2	3400	760	.22*	230	.07
WBR	1	1700	120	.07	80	.05
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.62		.86	

28. Ridge Route & Muirlands

Existing Counts						2011 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	103	.06*	.06		NBL	1	1700	.06*	.07
NBT	2	3400	153	.05	.08*		NBT	2	3400	.05	.08*
NBR	d	1700	88	.05	.10		NBR	d	1700	.05	.10
SBL	1	1700	151	.09*	.11*		SBL	1	1700	.09	.11*
SBT	2	3400	286	.08	.06		SBT	2	3400	.09*	.06
SBR	d	1700	79	.05	.03		SBR	d	1700	.05	.03
EBL	1	1700	54	.03*	.05		EBL	1	1700	.03*	.05
EBT	2	3400	325	.10	.30*		EBT	2	3400	.10	.32*
EBR	1	1700	39	.02	.04		EBR	1	1700	.02	.04
WBL	1	1700	135	.08	.05*		WBL	1	1700	.08	.05*
WBT	2	3400	853	.25*	.13		WBT	2	3400	.27*	.14
WBR	1	1700	97	.06	.06		WBR	1	1700	.06	.06
Clearance Interval			.05*		.05*		Clearance Interval			.05*	
Note: Assumes N/S Split Phasing						Note: Assumes N/S Split Phasing					
TOTAL CAPACITY UTILIZATION			.48		.59	TOTAL CAPACITY UTILIZATION			.50		.61

2011 With-Project						2015 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	111	.07*	.07		NBL	1	1700	.05	.06
NBT	2	3400	157	.05	.08*		NBT	2	3400	.08*	.08*
NBR	d	1700	91	.05	.10		NBR	d	1700	.07	.12
SBL	1	1700	143	.08	.10*		SBL	1	1700	.01	.06*
SBT	2	3400	320	.09*	.07		SBT	2	3400	.06*	.04
SBR	d	1700	82	.05	.03		SBR	d	1700	.02	.02
EBL	1	1700	57	.03*	.06		EBL	1	1700	.01*	.01
EBT	2	3400	348	.10	.33*		EBT	2	3400	.13	.41*
EBR	1	1700	41	.02	.04		EBR	1	1700	.02	.04
WBL	1	1700	158	.09	.05*		WBL	1	1700	.06	.06*
WBT	2	3400	881	.26*	.14		WBT	2	3400	.29*	.20
WBR	1	1700	99	.06	.07		WBR	1	1700	.04	.05
Clearance Interval			.05*		.05*		Clearance Interval			.05*	
Note: Assumes N/S Split Phasing						Note: Assumes N/S Split Phasing					
TOTAL CAPACITY UTILIZATION			.50		.61	TOTAL CAPACITY UTILIZATION			.49		.66

28. Ridge Route & Muirlands

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	80	.05	100	.06
NBT	2	3400	260	.08*	280	.08*
NBR	d	1700	130	.08	200	.12
SBL	1	1700	10	.01	100	.06*
SBT	2	3400	200	.06*	160	.05
SBR	d	1700	30	.02	40	.02
EBL	1	1700	20	.01*	20	.01
EBT	2	3400	440	.13	1400	.41*
EBR	1	1700	40	.02	70	.04
WBL	1	1700	110	.06	110	.06*
WBT	2	3400	970	.29*	670	.20
WBR	1	1700	70	.04	90	.05
Clearance Interval				.05*		.05*
Note: Assumes N/S Split Phasing						
TOTAL CAPACITY UTILIZATION			.49		.66	

29. El Toro & Muirlands

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	132	.04*	.08		NBL	2	3400	140	.04*	.08	
NBT	3	5100	754	.15	.29*		NBT	3	5100	799	.16	.31*	
NBR	1	1700	110	.06	.20		NBR	1	1700	117	.07	.21	
SBL	2	3400	217	.06	.226	.07*	SBL	2	3400	230	.07	.240	.07*
SBT	3	5100	1495	.29*	1043	.20	SBT	3	5100	1585	.31*	1106	.22
SBR	1	1700	351	.21	132	.08	SBR	1	1700	372	.22	140	.08
EBL	2	3400	157	.05*	71	.02	EBL	2	3400	166	.05*	75	.02
EBT	2	3400	254	.07	196	.06*	EBT	2	3400	269	.08	208	.06*
EBR	1	1700	161	.09	90	.05	EBR	1	1700	171	.10	95	.06
WBL	2	3400	270	.08	232	.07*	WBL	2	3400	286	.08	246	.07*
WBT	2	3400	638	.19*	313	.09	WBT	2	3400	676	.20*	332	.10
WBR	1	1700	254	.15	157	.09	WBR	1	1700	269	.16	166	.10
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.62		.54		TOTAL CAPACITY UTILIZATION			.65		.56	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	137	.04*	.294	.09	NBL	2	3400	120	.04*	.260	.08
NBT	3	5100	845	.17	1573	.31*	NBT	3	5100	1450	.28	1610	.32*
NBR	1	1700	117	.07	359	.21	NBR	1	1700	100	.06	420	.25
SBL	2	3400	232	.07	215	.06*	SBL	2	3400	220	.06	200	.06*
SBT	3	5100	1583	.31*	1136	.22	SBT	3	5100	1560	.31*	1030	.20
SBR	1	1700	378	.22	132	.08	SBR	1	1700	200	.12	50	.03
EBL	2	3400	168	.05*	84	.02	EBL	2	3400	90	.03*	80	.02
EBT	2	3400	258	.08	201	.06*	EBT	2	3400	280	.08	920	.27*
EBR	1	1700	173	.10	96	.06	EBR	1	1700	140	.08	350	.21
WBL	2	3400	307	.09	234	.07*	WBL	2	3400	280	.08	390	.11*
WBT	2	3400	661	.19*	333	.10	WBT	2	3400	720	.21*	500	.15
WBR	1	1700	245	.14	170	.10	WBR	1	1700	130	.08	180	.11
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.64		.55		TOTAL CAPACITY UTILIZATION			.64		.81	

29. El Toro & Muirlands

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	140	.04	250	.07
NBT	3	5100	1470	.29*	1570	.31*
NBR	1	1700	80	.05	420	.25
SBL	2	3400	270	.08*	170	.05*
SBT	3	5100	1560	.31	1040	.20
SBR	1	1700	180	.11	50	.03
EBL	2	3400	90	.03*	80	.02
EBT	2	3400	270	.08	910	.27*
EBR	1	1700	160	.09	360	.21
WBL	2	3400	290	.09	380	.11*
WBT	2	3400	710	.21*	510	.15
WBR	1	1700	100	.06	200	.12
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.66		.79	

30. Los Alisos & Muirlands

Existing Counts						2011 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	183	.11*	.11		NBL	1	1700	.11*	.12
NBT	3	5100	547	.11	.26*		NBT	3	5100	.11	.27*
NBR	d	1700	187	.11	.18		NBR	d	1700	.12	.19
SBL	1	1700	282	.17	.11*		SBL	1	1700	.18	.12*
SBT	3	5100	1045	.20*	.13		SBT	3	5100	.22*	.14
SBR	d	1700	252	.15	.10		SBR	d	1700	.16	.11
EBL	1	1700	111	.07*	.18*		EBL	1	1700	.07*	.19*
EBT	2	3400	399	.12	.20		EBT	2	3400	.12	.22
EBR	d	1700	155	.09	.14		EBR	d	1700	.10	.15
WBL	1	1700	177	.10	.08		WBL	1	1700	.11	.08
WBT	2	3400	523	.15*	.11*		WBT	2	3400	.16*	.12*
WBR	d	1700	109	.06	.09		WBR	d	1700	.07	.10
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.58		.71	TOTAL CAPACITY UTILIZATION			.61		.75

2011 With-Project						2015 No-Project					
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR
NBL	1	1700	176	.10*	.12		NBL	1	1700	.16*	.11
NBT	3	5100	609	.12	.28*		NBT	3	5100	.14	.32*
NBR	d	1700	203	.12	.19		NBR	d	1700	.05	.14
SBL	1	1700	312	.18	.12*		SBL	1	1700	.19	.16*
SBT	3	5100	1103	.22*	.15		SBT	3	5100	.26*	.16
SBR	d	1700	265	.16	.10		SBR	d	1700	.09	.18
EBL	1	1700	108	.06	.19*		EBL	1	1700	.11*	.24*
EBT	2	3400	424	.12*	.21		EBT	2	3400	.14	.22
EBR	d	1700	163	.10	.15		EBR	d	1700	.06	.12
WBL	1	1700	191	.11*	.08		WBL	1	1700	.12	.08
WBT	2	3400	552	.16	.12*		WBT	2	3400	.31*	.14*
WBR	d	1700	94	.06	.10		WBR	d	1700	.09	.21
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.60		.76	TOTAL CAPACITY UTILIZATION			.89		.91

30. Los Alisos & Muirlands

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	260	.15*	190	.11
NBT	3	5100	710	.14	1630	.32*
NBR	d	1700	90	.05	220	.13
SBL	1	1700	300	.18	280	.16*
SBT	3	5100	1290	.25*	840	.16
SBR	d	1700	200	.12	300	.18
EBL	1	1700	180	.11*	400	.24*
EBT	2	3400	500	.15	730	.21
EBR	d	1700	110	.06	190	.11
WBL	1	1700	210	.12	120	.07
WBT	2	3400	1010	.30*	470	.14*
WBR	d	1700	180	.11	340	.20
Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.86		.91	

31. Lake Forest & Rockfield

Existing Counts						2011 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	2	3400	419	.12*	.12		NBL	2	3400	444	.13*	.13
NBT	3	5100	1083	.21	.34*		NBT	3	5100	1148	.23	.37*
NBR	1	1700	166	.10	.40		NBR	1	1700	176	.10	.42
SBL	2	3400	81	.02	.06*		SBL	2	3400	86	.03	.06*
SBT	4	6800	1620	.25*	.19		SBT	4	6800	1717	.26*	.20
SBR	0	0	58		68		SBR	0	0	61		72
EBL	2	3400	17	.01	.05		EBL	2	3400	18	.01	.05
EBT	2	3400	52	.02*	.11*		EBT	2	3400	55	.02*	.11*
EBR	2	3400	109	.03	.13		EBR	2	3400	116	.03	.14
WBL	2	3400	570	.17*	.13*		WBL	2	3400	604	.18*	.14*
WBT	2	3400	329	.10	.06		WBT	2	3400	349	.10	.06
WBR	1	1700	47	.03	.07		WBR	1	1700	50	.03	.08
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.61		.69	TOTAL CAPACITY UTILIZATION			.64		.73	

2011 With-Project						2015 No-Project						
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	
NBL	2	3400	462	.14*	.12		NBL	2	3400	580	.17*	.18
NBT	3	5100	1159	.23	.37*		NBT	3	5100	1110	.22	.33*
NBR	1	1700	180	.11	.40		NBR	1	1700	150	.09	.28
SBL	2	3400	91	.03	.06*		SBL	2	3400	130	.04	.04*
SBT	4	6800	1754	.27*	.20		SBT	4	6800	1820	.28*	.19
SBR	0	0	51		84		SBR	0	0	70		130
EBL	2	3400	15	.00	.05		EBL	2	3400	70	.02*	.06
EBT	2	3400	49	.01*	.12*		EBT	2	3400	150	.04	.20*
EBR	2	3400	119	.04	.14		EBR	2	3400	210	.06	.09
WBL	2	3400	601	.18*	.14*		WBL	2	3400	450	.13	.12*
WBT	2	3400	358	.11	.06		WBT	2	3400	540	.16*	.06
WBR	1	1700	46	.03	.07		WBR	1	1700	110	.06	.09
Clearance Interval			.05*		.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.65		.74	TOTAL CAPACITY UTILIZATION			.68		.74	

31. Lake Forest & Rockfield

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	580	.17*	590	.17
NBT	3	5100	1120	.22	1750	.34*
NBR	1	1700	170	.10	470	.28
SBL	2	3400	130	.04	150	.04*
SBT	4	6800	1810	.28*	1190	.20
SBR	0	0	80		140	
EBL	2	3400	70	.02*	180	.05
EBT	2	3400	140	.04	680	.20*
EBR	2	3400	200	.06	300	.09
WBL	2	3400	440	.13	430	.13*
WBT	2	3400	530	.16*	210	.06
WBR	1	1700	110	.06	160	.09
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.68		.76	

32. Ridge Route & Rockfield

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0.5		44		26		NBL	0.5	47		28		
NBT	1.5	3400	33	.03*	25	.02*	NBT	1.5	3400	35	.04*	26	.02*
NBR	0		40		21		NBR	0		42		22	
SBL	0.5		144	.08*	155	.09*	SBL	0.5		153	.09*	164	.10*
SBT	1.5	3400	16	.01	26	.02	SBT	1.5	3400	17	.01	28	.02
SBR	d	1700	242	.14	113	.07	SBR	d	1700	257	.15	120	.07
EBL	1	1700	95	.06*	334	.20*	EBL	1	1700	101	.06*	354	.21*
EBT	2	3400	237	.08	953	.30	EBT	2	3400	251	.08	1010	.31
EBR	0	0	20		54		EBR	0	0	21		57	
WBL	1	1700	15	.01	19	.01	WBL	1	1700	16	.01	20	.01
WBT	2	3400	412	.14*	319	.13*	WBT	2	3400	437	.15*	338	.14*
WBR	0	0	59		138		WBR	0	0	63		146	
Right Turn Adjustment		SBR	.01*				Clearance Interval			.05*		.05*	
Clearance Interval			.05*		.05*								
TOTAL CAPACITY UTILIZATION			.37		.49		TOTAL CAPACITY UTILIZATION			.39		.52	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0.5		46		28		NBL	0.5		70		30	
NBT	1.5	3400	35	.04*	28	.02*	NBT	1.5	3400	40	.04*	20	.02*
NBR	0		42		22		NBR	0		40		10	
SBL	0.5		155	.09*	154	.09*	SBL	0.5		160	.09*	130	.08*
SBT	1.5	3400	18	.01	35	.02	SBT	1.5	3400	10	.01	20	.01
SBR	d	1700	278	.16	128	.08	SBR	d	1700	270	.16	120	.07
EBL	1	1700	97	.06*	342	.20*	EBL	1	1700	50	.03*	330	.19
EBT	2	3400	264	.08	1002	.31	EBT	2	3400	190	.06	1340	.41*
EBR	0	0	16		53		EBR	0	0	10		50	
WBL	1	1700	16	.01	20	.01	WBL	1	1700	10	.01	20	.01*
WBT	2	3400	414	.14*	327	.14*	WBT	2	3400	530	.18*	430	.16
WBR	0	0	58		143		WBR	0	0	80		130	
Right Turn Adjustment		SBR	.01*				Right Turn Adjustment		SBR	.05*			
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.39		.50		TOTAL CAPACITY UTILIZATION			.44		.57	

32. Ridge Route & Rockfield

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	0.5		60		30	
NBT	1.5	3400	40	.04*	20	.02*
NBR	0		40		10	
SBL	0.5		150	.09*	130	.08*
SBT	1.5	3400	10	.01	20	.01
SBR	d	1700	270	.16	130	.08
EBL	1	1700	70	.04*	340	.20
EBT	2	3400	200	.06	1330	.40*
EBR	0	0	10		40	
WBL	1	1700	10	.01	20	.01*
WBT	2	3400	520	.18*	420	.16
WBR	0	0	80		130	
Right Turn Adjustment		SBR	.04*			
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.44		.56	

33. El Toro & Rockfield

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	265	.08*	461	.14*	NBL	2	3400	281	.08*	489	.14*
NBT	4	6800	981	.14	1640	.24	NBT	4	6800	1040	.15	1738	.26
NBR	d	1700	124	.07	194	.11	NBR	d	1700	131	.08	206	.12
SBL	2	3400	115	.03	161	.05	SBL	2	3400	122	.04	171	.05
SBT	4	6800	1599	.25*	1214	.19*	SBT	4	6800	1695	.26*	1287	.21*
SBR	0	0	72		108		SBR	0	0	76		114	
EBL	2	3400	80	.02	352	.10	EBL	2	3400	85	.03	373	.11
EBT	2	3400	154	.05*	516	.15*	EBT	2	3400	163	.05*	547	.16*
EBR	f		227		292		EBR	f		241		310	
WBL	2	3400	366	.11*	336	.10*	WBL	2	3400	388	.11*	356	.10*
WBT	2	3400	235	.07	206	.06	WBT	2	3400	249	.07	218	.06
WBR	1	1700	97	.06	118	.07	WBR	1	1700	103	.06	125	.07
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.54		.63		TOTAL CAPACITY UTILIZATION			.55		.66	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	279	.08*	496	.15*	NBL	2	3400	220	.06*	310	.09*
NBT	4	6800	1064	.16	1735	.26	NBT	4	6800	1110	.16	1620	.24
NBR	d	1700	121	.07	221	.13	NBR	d	1700	40	.02	270	.16
SBL	2	3400	115	.03	157	.05	SBL	2	3400	200	.06	210	.06
SBT	4	6800	1697	.26*	1275	.21*	SBT	4	6800	1450	.23*	1480	.23*
SBR	0	0	67		119		SBR	0	0	130		70	
EBL	2	3400	94	.03	362	.11	EBL	2	3400	170	.05	480	.14
EBT	2	3400	155	.05*	530	.16*	EBT	2	3400	80	.02*	680	.20*
EBR	f		241		323		EBR	f		170		220	
WBL	2	3400	436	.13*	371	.11*	WBL	2	3400	530	.16*	290	.09*
WBT	2	3400	231	.07	212	.06	WBT	2	3400	160	.05	280	.08
WBR	1	1700	103	.06	113	.07	WBR	1	1700	110	.06	110	.06
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.57		.68		TOTAL CAPACITY UTILIZATION			.52		.66	

33. El Toro & Rockfield

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	220	.06*	310	.09*
NBT	4	6800	1110	.16	1560	.23
NBR	d	1700	40	.02	250	.15
SBL	2	3400	190	.06	220	.06
SBT	4	6800	1530	.24*	1490	.23*
SBR	0	0	120		70	
EBL	2	3400	160	.05	470	.14
EBT	2	3400	80	.02*	670	.20*
EBR	f		170		230	
WBL	2	3400	510	.15*	290	.09*
WBT	2	3400	160	.05	270	.08
WBR	1	1700	120	.07	100	.06
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.52		.66	

34. Los Alisos & Rockfield

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	179	.11*	203	.12*	NBL	1	1700	190	.11*	215	.13*
NBT	2	3400	666	.20	1177	.35	NBT	2	3400	706	.21	1248	.37
NBR	0	0	3		4		NBR	0	0	3		4	
SBL	1	1700	22	.01	28	.02	SBL	1	1700	23	.01	30	.02
SBT	2	3400	868	.39*	710	.30*	SBT	2	3400	920	.41*	753	.32*
SBR	0	0	448		300		SBR	0	0	475		318	
EBL	1.5		230	{.07}* [*]	403	{.13}* [*]	EBL	1.5		244	{.08}* [*]	427	{.14}* [*]
EBT	0.5	3400	13	.07	49	.13	EBT	0.5	3400	14	.08	52	.14
EBR	1	1700	154	.09	268	.16	EBR	1	1700	163	.10	284	.17
WBL	0	0	11		4		WBL	0	0	12		4	
WBT	1	1700	36	.03*	21	.01*	WBT	1	1700	38	.03*	22	.02*
WBR	d	1700	55	.03	40	.02	WBR	d	1700	58	.03	42	.02
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.65		.61		TOTAL CAPACITY UTILIZATION			.68		.66	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	1	1700	179	.11*	211	.12*	NBL	1	1700	210	.12*	310	.18*
NBT	2	3400	716	.21	1267	.37	NBT	2	3400	880	.26	1410	.42
NBR	0	0	3		4		NBR	0	0	10		10	
SBL	1	1700	19	.01	35	.02	SBL	1	1700	10	.01	10	.01
SBT	2	3400	889	.41*	769	.32*	SBT	2	3400	980	.46*	910	.34*
SBR	0	0	512		325		SBR	0	0	590		250	
EBL	1.5		243	{.08}* [*]	421	{.14}* [*]	EBL	1.5		230	{.09}* [*]	600	{.19}* [*]
EBT	0.5	3400	14	.08	57	.14	EBT	0.5	3400	90	.09	40	.19
EBR	1	1700	157	.09	276	.16	EBR	1	1700	270	.16	310	.18
WBL	0	0	12		4		WBL	0	0	20		20	
WBT	1	1700	36	.03*	22	.02*	WBT	1	1700	100	.07*	70	.05*
WBR	d	1700	61	.04	42	.02	WBR	d	1700	40	.02	20	.01
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.68		.65		TOTAL CAPACITY UTILIZATION			.79		.81	

34. Los Alisos & Rockfield

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	220	.13*	300	.18*
NBT	2	3400	880	.26	1440	.43
NBR	0	0	10		10	
SBL	1	1700	10	.01	20	.01
SBT	2	3400	990	.46*	920	.34*
SBR	0	0	570		240	
EBL	1.5		230	{.09}* [*]	560	{.18}* [*]
EBT	0.5	3400	90	.09	40	.18
EBR	1	1700	250	.15	310	.18
WBL	0	0	20		20	
WBT	1	1700	100	.07*	70	.05*
WBR	d	1700	40	.02	20	.01
Clearance Interval				.05*		.05*
TOTAL CAPACITY UTILIZATION			.80		.80	

35. Lake Forest & I-5 NB

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	3	5100	1262	.25	2353	.46*	NBT	3	5100	1338	.26	2494	.49*
NBR	0	0	0		0		NBR	0	0		0		
SBL	0	0	0		0		SBL	0	0		0		
SBT	3	5100	1264	.25*	1355	.27	SBT	3	5100	1340	.26*	1436	.28
SBR	f		906		783		SBR		960		830		
EBL	0	0	0		0		EBL	0	0		0		
EBT	0	0	0		0		EBT	0	0		0		
EBR	0	0	0		0		EBR	0	0		0		
WBL	2	3400	464	.14*	193	.06*	WBL	2	3400	492	.14*	205	.06*
WBT	0	0	0		0		WBT	0	0		0		
WBR	2	3400	520	.15	375	.11	WBR	2	3400	551	.16	397	.12
Right Turn Adjustment		WBR	.01*		WBR	.05*	Right Turn Adjustment		WBR	.02*		WBR	.06*
Clearance Interval			.05*			.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.45		.62		TOTAL CAPACITY UTILIZATION			.47		.66	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	3	5100	1379	.27*	2469	.48*	NBT	3	5100	1670	.33*	2380	.47*
NBR	0	0	0		0		NBR	0	0		0		
SBL	0	0	0		0		SBL	0	0		0		
SBT	3	5100	1343	.26	1435	.28	SBT	3	5100	1060	.21	1370	.27
SBR	f		995		834		SBR		1400		1000		
EBL	0	0	0		0		EBL	0	0		0		
EBT	0	0	0		0		EBT	0	0		0		
EBR	0	0	0		0		EBR	0	0		0		
WBL	2	3400	493	.15*	201	.06*	WBL	2	3400	520	.15*	200	.06*
WBT	0	0	0		0		WBT	0	0		0		
WBR	2	3400	537	.16	394	.12	WBR	2	3400	640	.19	420	.12
Right Turn Adjustment		WBR	.01*		WBR	.06*	Right Turn Adjustment		WBR	.04*		WBR	.06*
Clearance Interval			.05*			.05*	Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.48		.65		TOTAL CAPACITY UTILIZATION			.57		.64	

35. Lake Forest & I-5 NB

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	0	0	0		0	
NBT	3	5100	1710	.34*	2410	.47*
NBR	0	0	0		0	
SBL	0	0	0		0	
SBT	3	5100	1060	.21	1370	.27
SBR	f		1370		1000	
EBL	0	0	0		0	
EBT	0	0	0		0	
EBR	0	0	0		0	
WBL	2	3400	530	.16*	190	.06*
WBT	0	0	0		0	
WBR	2	3400	620	.18	410	.12
Right Turn Adjustment		WBR	.02*		WBR	.06*
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.57		.64	

36. Lake Forest & I-5/Carlota

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	4	6800	560	.09	1177	.19*	NBT	4	6800	594	.10	1248	.20*
NBR	0	0	58		113		NBR	0	0	61		120	
SBL	2	3400	261	.08	258	.08*	SBL	2	3400	277	.08	273	.08*
SBT	3	5100	901	.18*	546	.11	SBT	3	5100	955	.19*	579	.11
SBR	f		446		399		SBR	f		473		423	
EBL	2.5		848	.17	1383		EBL	2.5		899	.18	1466	
EBT	1.5	6800	263	.15*	615	.29*	EBT	1.5	6800	279	.16*	652	.31*
EBR	1	1700	619	.36	311	.18	EBR	1	1700	656	.39	330	.19
WBL	1	1700	118	.07*	145	.09*	WBL	1	1700	125	.07*	154	.09*
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	2	3400	145	.04	302	.09	WBR	2	3400	154	.05	320	.09
Right Turn Adjustment		EBR	.21*				Right Turn Adjustment	EBR	.23*				
Clearance Interval			.05*				Clearance Interval		.05*			.05*	
TOTAL CAPACITY UTILIZATION			.66		.70		TOTAL CAPACITY UTILIZATION			.70		.73	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	4	6800	599	.10	1261	.20*	NBT	4	6800	640	.10*	1060	.17*
NBR	0	0	61		118		NBR	0	0	50		80	
SBL	2	3400	281	.08	273	.08*	SBL	2	3400	330	.10*	380	.11*
SBT	3	5100	950	.19*	579	.11	SBT	3	5100	770	.15	650	.13
SBR	f		478		417		SBR	f		550		570	
EBL	2.5		922	.18	1441		EBL	2.5		940		1840	
EBT	1.5	6800	275	.16*	642	.31*	EBT	1.5	6800	430	.20*	880	.40*
EBR	1	1700	611	.36	300	.18	EBR	1	1700	620	.36	350	.21
WBL	1	1700	121	.07*	153	.09*	WBL	1	1700	120	.07*	130	.08*
WBT	0	0	0		0		WBT	0	0	0		0	
WBR	2	3400	155	.05	317	.09	WBR	2	3400	170	.05	400	.12
Right Turn Adjustment		EBR	.20*				Right Turn Adjustment	EBR	.12*				
Clearance Interval			.05*				Clearance Interval		.05*			.05*	
TOTAL CAPACITY UTILIZATION			.67		.73		TOTAL CAPACITY UTILIZATION			.64		.81	

36. Lake Forest & I-5/Carlota

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	0	0	0		0	
NBT	4	6800	630	.10*	1080	.17*
NBR	0	0	40		70	
SBL	2	3400	330	.10*	370	.11*
SBT	3	5100	800	.16	650	.13
SBR	f		540		570	
EBL	2.5		970		1870	
EBT	1.5	6800	430	.21*	900	.41*
EBR	1	1700	590	.35	320	.19
WBL	1	1700	120	.07*	130	.08*
WBT	0	0	0		0	
WBR	2	3400	170	.05	390	.11
Right Turn Adjustment		EBC	.11*			
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.64		.82	

37. Paseo De Valencia & Carlota

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	179	.05*	165	.05	NBL	2	3400	190	.06*	175	.05
NBT	2	3400	33	.01	124	.06*	NBT	2	3400	35	.02	131	.07*
NBR	0	0	15		88		NBR	0	0	16		93	
SBL	2	3400	702	.21	873	.26*	SBL	2	3400	744	.22	925	.27*
SBT	2	3400	603	.25*	370	.11	SBT	2	3400	639	.26*	392	.12
SBR	0	0	231		20		SBR	0	0	245		21	
EBL	2	3400	116	.03*	219	.06	EBL	2	3400	123	.04*	232	.07
EBT	2	3400	171	.05	591	.17*	EBT	2	3400	181	.05	626	.18*
EBR	1	1700	85	.05	452	.27	EBR	1	1700	90	.05	479	.28
WBL	1	1700	7	.00	18	.01*	WBL	1	1700	7	.00	19	.01*
WBT	2	3400	298	.09*	227	.07	WBT	2	3400	316	.09*	241	.07
WBR	1	1700	446	.26	491	.29	WBR	1	1700	473	.28	520	.31
Clearance Interval			.05*		.05*		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.47		.55		TOTAL CAPACITY UTILIZATION			.50		.58	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	2	3400	185	.05*	175	.05	NBL	2	3400	160	.05	220	.06
NBT	2	3400	42	.02	131	.07*	NBT	2	3400	20	.01*	100	.06*
NBR	0	0	16		93		NBR	0	0	40	.02	270	.16
SBL	2	3400	750	.22	906	.27*	SBL	2	3400	920	.27*	1000	.29*
SBT	2	3400	658	.27*	395	.12	SBT	2	3400	590	.18	480	.15
SBR	0	0	246		26		SBR	0	0	10		30	
EBL	2	3400	119	.04*	224	.07	EBL	2	3400	90	.03*	440	.13*
EBT	2	3400	172	.05	649	.19*	EBT	2	3400	230	.07	690	.20
EBR	1	1700	89	.05	473	.28	EBR	1	1700	110	.06	680	.40
WBL	1	1700	7	.00	19	.01*	WBL	1	1700	30	.02	50	.03
WBT	2	3400	313	.09*	231	.07	WBT	2	3400	470	.14*	360	.11*
WBR	1	1700	473	.28	516	.30	WBR	1	1700	510	.30	530	.31
Clearance Interval			.05*		.05*		Right Turn Adjustment			Multi		.11*	
TOTAL CAPACITY UTILIZATION			.50		.59		Clearance Interval			.05*		.05*	
TOTAL CAPACITY UTILIZATION			.50		.59		TOTAL CAPACITY UTILIZATION			.50		.75	

37. Paseo De Valencia & Carlota

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	2	3400	160	.05	220	.06
NBT	2	3400	20	.01*	100	.06*
NBR	0	0	40	.02	270	.16
SBL	2	3400	920	.27*	960	.28*
SBT	2	3400	610	.18	480	.15
SBR	0	0	10		30	
EBL	2	3400	80	.02*	440	.13*
EBT	2	3400	240	.07	700	.21
EBR	1	1700	110	.06	670	.39
WBL	1	1700	30	.02	40	.02
WBT	2	3400	460	.14*	360	.11*
WBR	1	1700	500	.29	520	.31
Right Turn Adjustment				Multi		.11*
Clearance Interval			.05*			.05*
TOTAL CAPACITY UTILIZATION			.49		.74	

38. El Toro & Bridger/I-5 NB

Existing Counts								2011 No-Project								
	LANES	CAPACITY	AM VOL	PK V/C	HOUR	PM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	PM VOL	PK V/C
NBL	1	1700	56	.03*		87	.05		NBL	1	1700	59	.03*		92	.05
NBT	2.5	6800	843	{.19}		1486	{.31}*		NBT	2.5	6800	894	{.21}		1575	{.32}*
NBR	1.5		707			898			NBR	1.5		749			952	
SBL	0	0	0			0			SBL	0	0	0			0	
SBT	5	8500	2261	.28*		1808	.23		SBT	5	8500	2397	.30*		1916	.24
SBR	0	0	121			113			SBR	0	0	128			120	
EBL	1	1700	33	.02*		123	.07*		EBL	1	1700	35	.02*		130	.08*
EBT	1	1700	7	.00		4	.00		EBT	1	1700	7	.00		4	.00
EBR	1	1700	171	.10		199	.12		EBR	1	1700	181	.11		211	.12
WBL	1.5		429			481			WBL	1.5		455			510	
WBT	0	5100	66	{.16}*		63	.23*		WBT	0	5100	70	{.16}*		67	.25*
WBR	1.5		464			717			WBR	1.5		492			760	
Right Turn Adjustment		EBR	.06*						Right Turn Adjustment		EBR	.07*				
Clearance Interval			.05*						Clearance Interval			.05*				.05*
Note: Assumes E/W Split Phasing									Note: Assumes E/W Split Phasing							
TOTAL CAPACITY UTILIZATION			.60			.66			TOTAL CAPACITY UTILIZATION			.63			.70	

2011 With-Project								2015 No-Project								
	LANES	CAPACITY	AM VOL	PK V/C	HOUR	PM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR	PM VOL	PK V/C
NBL	1	1700	59	.03*		92	.05		NBL	1	1700	60	.04*		160	.09*
NBT	2.5	6800	899	{.20}		1577	{.31}*		NBT	2.5	6800	1050	{.24}		1440	{.32}
NBR	1.5		749			969			NBR	1.5		910			1120	
SBL	0	0	0			0			SBL	0	0	0			0	
SBT	5	8500	2447	.30*		1923	.24		SBT	5	8500	2310	.28*		1960	.24*
SBR	0	0	128			120			SBR	0	0	80			90	
EBL	1	1700	35	.02*		130	.08*		EBL	1	1700	40	.02*		110	.06*
EBT	1	1700	7	.00		4	.00		EBT	1	1700	10	.01		10	.01
EBR	1	1700	181	.11		211	.12		EBR	1	1700	150	.09		220	.13
WBL	1.5		454			513			WBL	1.5		520			480	
WBT	0	5100	70	{.16}*		67	.25*		WBT	0	5100	80	{.22}*		60	.23*
WBR	1.5		492			764			WBR	1.5		630			670	
Right Turn Adjustment		EBR	.06*						Right Turn Adjustment		EBR	.03*				
Clearance Interval			.05*						Clearance Interval			.05*			.05*	
Note: Assumes Right-Turn Overlap for EBR									Note: Assumes Right-Turn Overlap for EBR							
TOTAL CAPACITY UTILIZATION			.62			.69			TOTAL CAPACITY UTILIZATION			.64			.67	

38. El Toro & Bridger/I-5 NB

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	1	1700	60	.04*	160	.09*
NBT	2.5	6800	1060	{.25}	1350	{.31}
NBR	1.5		910		1120	
SBL	0	0	0		0	
SBT	5	8500	2360	.29*	1980	.24*
SBR	0	0	80		90	
EBL	1	1700	40	.02*	110	.06*
EBT	1	1700	10	.01	10	.01
EBR	1	1700	150	.09	220	.13
WBL	1.5		520		480	
WBT	0	5100	80	{.21}* Clearance Interval	60	.23*
WBR	1.5		620		680	
Right Turn Adjustment		EBR	.03*			
Clearance Interval			.05*			.05*
Note: Assumes Right-Turn Overlap for EBR						
TOTAL CAPACITY UTILIZATION			.64		.67	

39. El Toro & Avd Carlota

Existing Counts						2011 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	4	6800	949	.14	1759	.26*	NBT	4	6800	1006	.15*	1865	.27*
NBR	d	1700	13	.01	49	.03	NBR	d	1700	14	.01	52	.03
SBL	2	3400	115	.03	578	.17*	SBL	2	3400	122	.04*	613	.18*
SBT	3	5100	890	.17*	721	.14	SBT	3	5100	943	.18	764	.15
SBR	1	1700	620	.36	241	.14	SBR	1	1700	657	.39	255	.15
EBL	1.5		592	.17*	829		EBL	1.5		628	.18*	879	
EBT	1.5	5100	227	.13	589	.28*	EBT	1.5	5100	241	.14	624	.29*
EBR	1	1700	118	.07	84	.05	EBR	1	1700	125	.07	89	.05
WBL	1	1700	22	.01	46	.03	WBL	1	1700	23	.01	49	.03
WBT	1	1700	100	.06*	90	.05*	WBT	1	1700	106	.06*	95	.06*
WBR	1	1700	310	.18	390	.23	WBR	1	1700	329	.19	413	.24
Right Turn Adjustment	Multi		.11*		WBR	.01*	Right Turn Adjustment	Multi	.11*				
Clearance Interval			.05*			.05*	Clearance Interval		.05*		.05*		
Note: Assumes E/W Split Phasing						Note: Assumes E/W Split Phasing							
Note: Assumes Right-Turn Overlap for SBR WBR						Note: Assumes Right-Turn Overlap for SBR WBR							
TOTAL CAPACITY UTILIZATION			.56		.82		TOTAL CAPACITY UTILIZATION			.59		.85	

2011 With-Project						2015 No-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR		LANES	CAPACITY	AM VOL	PK V/C	HOUR		
NBL	0	0	0		0		NBL	0	0		0		
NBT	4	6800	1010	.15*	1861	.27*	NBT	4	6800	880	.13	1590	.23*
NBR	d	1700	16	.01	61	.04	NBR	d	1700	10	.01	50	.03
SBL	2	3400	123	.04*	613	.18*	SBL	2	3400	100	.03	320	.09*
SBT	3	5100	950	.19	788	.15	SBT	3	5100	960	.19*	780	.15
SBR	1	1700	656	.39	248	.15	SBR	1	1700	800	.47	710	.42
EBL	1.5		615	.18*	886		EBL	1.5		850	.25*	900	
EBT	1.5	5100	230	.14	618	.29*	EBT	1.5	5100	250	.15	800	.33*
EBR	1	1700	148	.09	93	.05	EBR	1	1700	170	.10	140	.08
WBL	1	1700	10	.01	59	.03	WBL	1	1700	90	.05	40	.02
WBT	1	1700	104	.06*	90	.05*	WBT	1	1700	130	.08*	80	.05*
WBR	1	1700	336	.20	422	.25	WBR	1	1700	240	.14	640	.38
Right Turn Adjustment	Multi		.12*		WBR	.02*	Right Turn Adjustment	Multi	.04*		WBR	.24*	
Clearance Interval			.05*			.05*	Clearance Interval		.05*			.05*	
Note: Assumes E/W Split Phasing						Note: Assumes E/W Split Phasing							
Note: Assumes Right-Turn Overlap for SBR WBR						Note: Assumes Right-Turn Overlap for SBR WBR							
TOTAL CAPACITY UTILIZATION			.60		.86		TOTAL CAPACITY UTILIZATION			.61		.99	

39. El Toro & Avd Carlota

2015 With-Project						
	LANES	CAPACITY	AM PK VOL	HOUR V/C	PM PK VOL	HOUR V/C
NBL	0	0	0		0	
NBT	4	6800	880	.13	1590	.23*
NBR	d	1700	10	.01	50	.03
SBL	2	3400	100	.03	320	.09*
SBT	3	5100	960	.19*	790	.15
SBR	1	1700	790	.46	710	.42
EBL	1.5		860	.25*	820	
EBT	1.5	5100	260	.15	840	.33*
EBR	1	1700	160	.09	150	.09
WBL	1	1700	100	.06	40	.02
WBT	1	1700	120	.07*	90	.05*
WBR	1	1700	240	.14	640	.38
Right Turn Adjustment	Multi		.04*	WBR	.24*	
Clearance Interval			.05*			.05*
Note: Assumes E/W Split Phasing						
Note: Assumes Right-Turn Overlap for SBR WBR						
TOTAL CAPACITY UTILIZATION			.60		.99	

40. Portola & Rancho

2011 With-Project							2015 With-Project							
	LANES	CAPACITY	AM VOL	PK V/C	HOUR	PM VOL	PK V/C	HOUR	AM VOL	PK V/C	HOUR	PM VOL	PK V/C	HOUR
NBL	2	3400	899	.26*		472	.14*		NBL	2	3400	1030	.30*	
NBT	3	5100	1689	.33		1048	.21		NBT	3	5100	1320	.26	
NBR	0	0	0			0			NBR	0	0	0		
SBL	0	0	0			0			SBL	0	0	0		
SBT	3	5100	849	.17*		1688	.33*		SBT	3	5100	820	.16*	
SBR	d	1700	178	.10		18	.01		SBR	d	1700	150	.09	
EBL	1.5		31	.01*		49	.03*		EBL	1.5		60	.02*	
EBT	0	5100	0			0			EBT	0	5100	0		
EBR	1.5		287			926	.27		EBR	1.5		310		
WBL	0	0	0			0			WBL	0	0	0		
WBT	0	0	0			0			WBT	0	0	0		
WBR	0	0	0			0			WBR	0	0	0		
Right Turn Adjustment						EBR	.13*		Right Turn Adjustment				EBR	.11*
Clearance Interval			.05*				.05*		Clearance Interval			.05*		.05*
TOTAL CAPACITY UTILIZATION			.49			.68			TOTAL CAPACITY UTILIZATION			.53		.66



MEMORANDUM

TO: Cheryl Kuta, Planning Manager, City of Lake Forest

FROM: Krys Saldivar, Austin-Foust Associates, Inc.

DATE: August 23, 2010

SUBJECT: **Lake Forest Sports Park and Recreation Center Alternative 7 Sensitivity Analysis**

Austin-Foust Associates, Inc. (AFA) has conducted a sensitivity analysis that assumes alternative land uses in the Opportunities Study Area, specifically for Alternative 7 versus Alternative 8 that is currently in the traffic study. The attached information presents the pages that are revised in the traffic study.

When you have reviewed the information we would be pleased to discuss the results by phone or meet in person.

Thank you for the opportunity to assist the City of Lake Forest in this important planning effort.

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Short-Term (Year 2015 Cumulative) Peak Hour Freeway/Tollway Mainline Levels of Service

Short-term (year 2015 cumulative) with-project AM and PM freeway mainline peak hour volumes and V/C ratios for with and without project are summarized in Table 14. Based on the peak hour mainline performance criteria and impact thresholds established for the analysis, no freeway mainline segment is forecast to be significantly impacted by the proposed project land uses under year short-term (year 2015 cumulative) conditions (i.e., the project does not cause LOS “F” conditions or contributes more than a .03 V/C to an already deficient LOS “F” condition). It should be noted that the LOS thresholds and significance criteria used here are from the CMP and do not necessarily represent Caltrans policy.

SPECIAL ISSUES

This section summarizes the special issues that were evaluated as part of the traffic analysis for the proposed Lake Forest Sports Park and Recreation Center project. The special issues addressed in this section deal with the following subject areas:

- **Signal Warrant Analysis** – This special issue presents a preliminary analysis for future traffic signal needs at the major entrance proposed on Rancho Parkway.
- **Alternative 7 Sensitivity Analysis** – This special issue presents an analysis of the proposed project assuming different background conditions than previously analyzed in this report.

Signal Warrant Analysis

This section includes a preliminary analysis of the signalization needs for the major entrance to the sports park proposed on the planned extension of Rancho Parkway to Portola Parkway. It is considered preliminary since detailed site planning for the sports park has yet to be completed at this time.

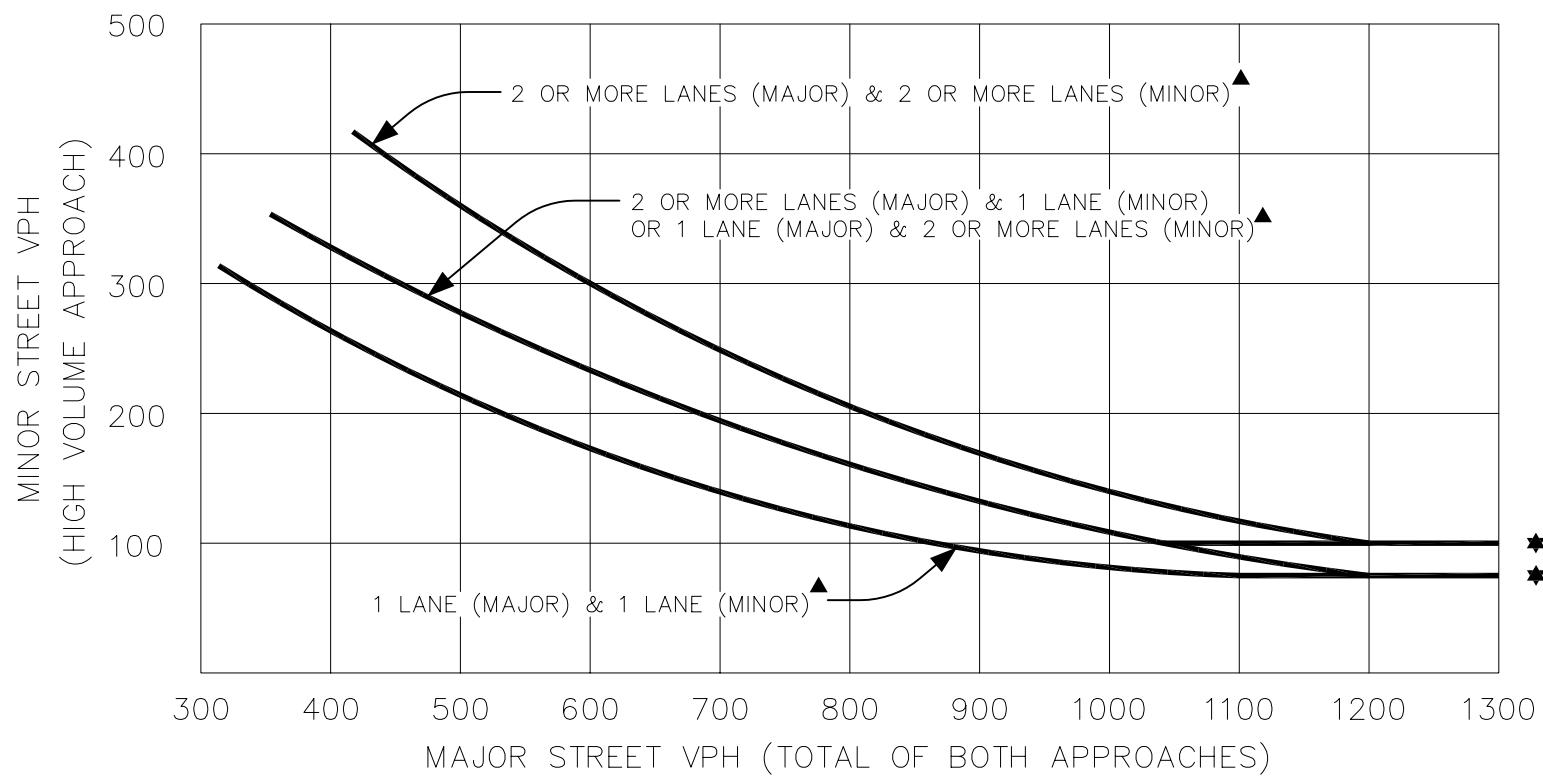
Traffic signal warrants based on peak hour volumes as adopted by the Federal Highway Administration and Caltrans were used here to determine the need for signalization. In applying this warrant, the volumes of both the major and minor street must meet or exceed those shown on the curves in Figure 30 for conditions when the speed on the major street is 40 (mph) or higher which is expected to be experienced by Rancho Parkway.

Table 14

SHORT-TERM (YEAR 2015 CUMULATIVE) FREEWAY/TOLLWAY MAINLINE LOS SUMMARY

Location	Direction	Lanes	Peak Hour Capacity	No-Project						With-Project					
				AM Peak Hour			PM Peak Hour			AM Peak Hour			PM Peak Hour		
				Volume	V/C	LOS									
I-5 n/o Lake Forest	Northbound	8+2H	19,500	15,508	.80	D	12,651	.65	C	15,508	.80	D	12,682	.65	C
	Southbound	8+2H	19,500	11,806	.61	C	15,990	.82	D	11,844	.61	C	15,996	.82	D
I-5 n/o El Toro	Northbound	6+2H	15,500	15,527	1.00	E	12,269	.79	D	15,553	1.00	E	12,275	.79	D
	Southbound	6+2H	15,500	10,546	.68	C	14,546	.94	E	10,559	.68	C	14,546	.94	E
I-5 n/o Alicia	Northbound	4+1H	9,600	14,414	1.50	F	11,439	1.19	F	14,414	1.50	F	11,454	1.19	F
	Southbound	4+1H	9,600	9,661	1.01	F	14,370	1.50	F	9,661	1.01	F	14,370	1.50	F
SR-241 n/o Lake Forest	Northbound	3	6,000	5,079	.85	D	2,419	.40	B	5,079	.85	D	2,419	.40	B
	Southbound	3	6,000	2,027	.34	B	4,522	.75	D	2,027	.34	B	4,533	.76	D
SR-241 n/o Portola East	Northbound	3	6,000	4,957	.83	D	1,836	.31	B	4,957	.83	D	1,836	.31	B
	Southbound	3	6,000	1,387	.23	A	4,265	.71	C	1,387	.23	A	4,265	.71	C
SR-241 n/o Los Alisos	Northbound	3	6,000	4,863	.81	D	1,636	.27	A	4,863	.81	D	1,636	.27	A
	Southbound	3	6,000	1,195	.20	A	4,022	.67	C	1,195	.20	A	4,022	.67	C

Abbreviations: H – high-occupancy vehicle lane
 LOS – level of service
 V/C – volume/capacity ratio



- ▲ NOTE: THESE CURVES ARE RECOMMENDED FOR USE IN AREAS WHERE THE POSTED SPEED LIMIT ON THE MAJOR STREET IS 40 MPH OR HIGHER.
- ▲ NOTE: 100 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH TWO OR MORE LANES, AND 75 VPH APPLIES AS THE LOWER THRESHOLD VOLUME FOR A MINOR STREET APPROACH WITH ONE LANE.

Figure 30

PEAK HOUR SIGNAL WARRANTS
(HIGHER SPEEDS)

Determining the major street approach for the signal warrant involves calculating the number of vehicles approaching the intersection on both major street legs. The highest total volume for either the continuous east and west approach or the north and south approach during either AM and PM is determined to be the major street approach for both peak hours. The minor street peak hour signal warrant volume is the number of peak hour vehicles approaching the intersection on only the highest volume leg. The highest volume for either the AM or PM determines the minor approach for both peak hours

The signal warrant analysis has been carried out for the intersection of the main entrance to the proposed sports park on Rancho Parkway. Since the worst-case volume on the major street (Rancho Parkway) is forecast to be over 1,500 vehicles in the PM peak hour under year 2015 cumulative with-project conditions, only a minimum volume of 75 vehicles on the minor street (proposed project access driveway) is required to determine if signal warrants are met. This indicates that of the total forecast volume in the PM peak hour of 503 only 15 percent of vehicles arriving and leaving the sports park would meet signal warrants. It is likely that this will occur and based on the application of the warrant, traffic signals should be installed at the proposed access intersection at Rancho Parkway under year 2015 cumulative with-project conditions. However, signals typically are not installed until warrants are met.

Alternative 7 Sensitivity Analysis

Alternative 7 was adopted in 2008 and included five participating landowners in the OSA. One of the five landowners, Site 1 Shea/Baker, at the start of this study had not yet signed a development agreement and was thus not a formal participant in the OSA. Therefore, the main body of this report assumed the current General Plan land uses in Shea/Baker which are mostly business park uses compared to residential use in Alternative 7. Buildout (2030) land use and trip generation for the OSA sites under cumulative conditions with current General Plan including non-participating Sites 1, 4 and 7, are summarized in Table 15 along with Alternative 7. As seen in Table 15, the trip generation is the highest under the Current General Plan and lowest under Alternative 7.

The proposed Sports Park and Recreation Center project is analyzed here for Alternative 7 and compared to the current General Plan results under short-range (year 2015 cumulative) conditions in this report. The 2011 analysis previously presented assumed only ambient growth in the area (i.e., no growth assumed in the OSA). Therefore the results are also applicable for Alternative 7.

Table 15

**BUILDOUT LAND USE AND TRIP GENERATION SUMMARY
(CUMULATIVE CONDITIONS FOR OSA SITES 1-7 AND 9)**

Land Use	Units	AM Peak Hour			PM Peak Hour			ADT
		In	Out	Total	In	Out	Total	
Alternative 7								
Single Family Detached	1,530 DU	290	857	1,147	994	551	1,545	14,642
Condominium	1,793 DU	304	898	1,202	807	591	1,398	14,613
Apartment	1,415 DU	141	581	722	566	311	877	9,509
Commercial (EQ)	160 TSF	167	107	274	475	515	990	11,388
Community Facility	44 TSF	36	7	43	100	108	208	2,002
Government Facility	44 TSF	87	11	98	39	87	126	1,228
Park	44 Acre	0	0	0	1	1	2	71
Business Park	2,041.7 TSF	2,450	470	2,920	613	2,021	2,634	26,052
Sports Park	63 Acre	1	0	1	214	258	472	3,389
Total Alternative 7		3,476	2,931	6,407	3,809	4,443	8,252	82,894
Current General Plan								
Single Family Detached	641 DU	122	359	481	415	232	647	6,134
Condominium	367 DU	62	184	246	165	121	286	2,991
Apartment	915 DU	91	376	467	366	201	567	6,149
Commercial (EQ)	780.52 TSF	513	328	841	1,464	1,586	3,050	35,062
Community Facility	44 TSF	36	7	43	100	108	208	2,002
Government Facility	44 TSF	87	11	98	39	87	126	1,228
Open Space	15.7 Acre	0	0	0	0	0	0	0
Park	36 Acre	0	0	0	0	0	0	59
Business Park	6,637.32 TSF	7,964	1,526	9,491	1,992	6,571	8,563	84,692
Sports Park	13 Acre	0	0	0	44	53	97	699
Total Current General Plan		8,875	2,791	11,667	4,585	8,959	13,544	139,016

The growth assumed in the OSA between existing to year 2015 is the same between Alternative 7 and the current General Plan assumptions which is around 25 percent. Also should the project require mitigation measures for Alternative 7 under year 2015 conditions, reference will be first made of any LFTM improvement.

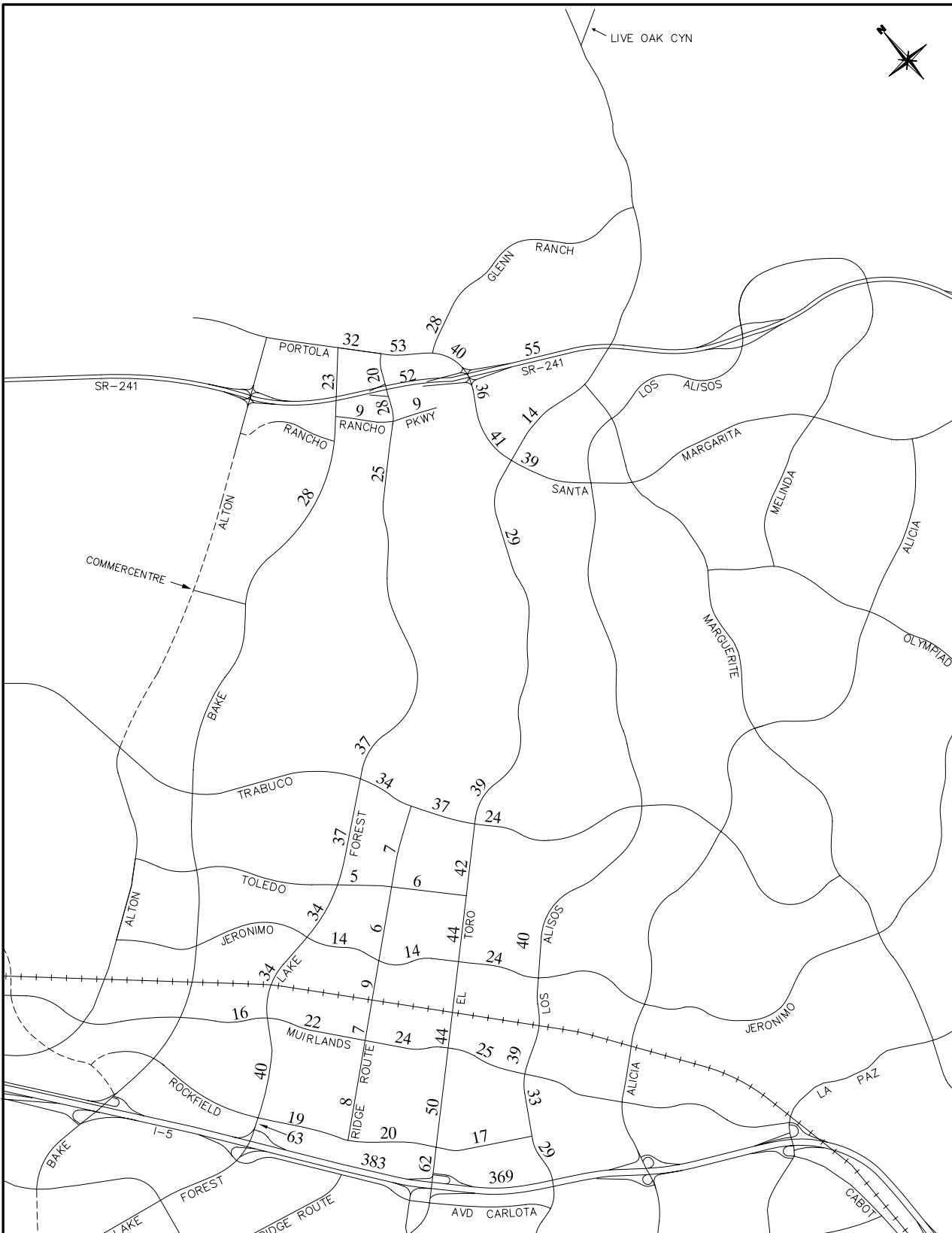
In the cumulative analysis year 2015, the ADT volumes for Alternative 7 under no-project and with-project conditions with buildup of the proposed project (year 2015) are presented in Figures 31 and 32. No significant changes in ADT volume can be seen when compared to the previous ADT volumes under current General Plan conditions. As such we would expect little change in the peak hour ICU results as well.

The future year 2015 ICU values and corresponding LOS for the intersections previously illustrated in Figure 24 and analyzed here are summarized in Table 16 (see Appendix C for detailed ICU calculations). Based on the peak hour intersection performance criteria and impact thresholds established for the analysis, one intersection, Lake Forest Drive and Rancho Parkway, within the study area is significantly impacted by the proposed project land uses under short-term (year 2015 cumulative) Alternative 7 conditions (no-project PM peak hour ICU of .67 increases to .92 for with-project) which is consistent with the previous 2015 results.

As mentioned before, the LFTM Program includes improvements at the intersection of Lake Forest Drive and Rancho Parkway which would mitigate the project impact resulting from the Sports Park/Recreation Center project. The analyses in this report for both Alternative 7 and current General Plan conditions in the OSA indicate that the improvements should be implemented no later than year 2015. Since the improvements listed in LFTM for the intersection of Lake Forest Drive and Rancho Parkway exceed what are required to mitigate the impacts of the Sports Park/Recreation Center project, the EIR will include a mitigation measure to ensure that the minimum improvements necessary to accommodate the proposed project (a second eastbound through lane on Rancho Parkway) will be constructed no later than year 2015 for either Alternative 7 and current General Plan conditions in the OSA.

FINDINGS AND CONCLUSIONS

The proposed project involves the development of a sports park and recreation center oriented toward youth, adult, and senior recreation activities. The development of the proposed project has

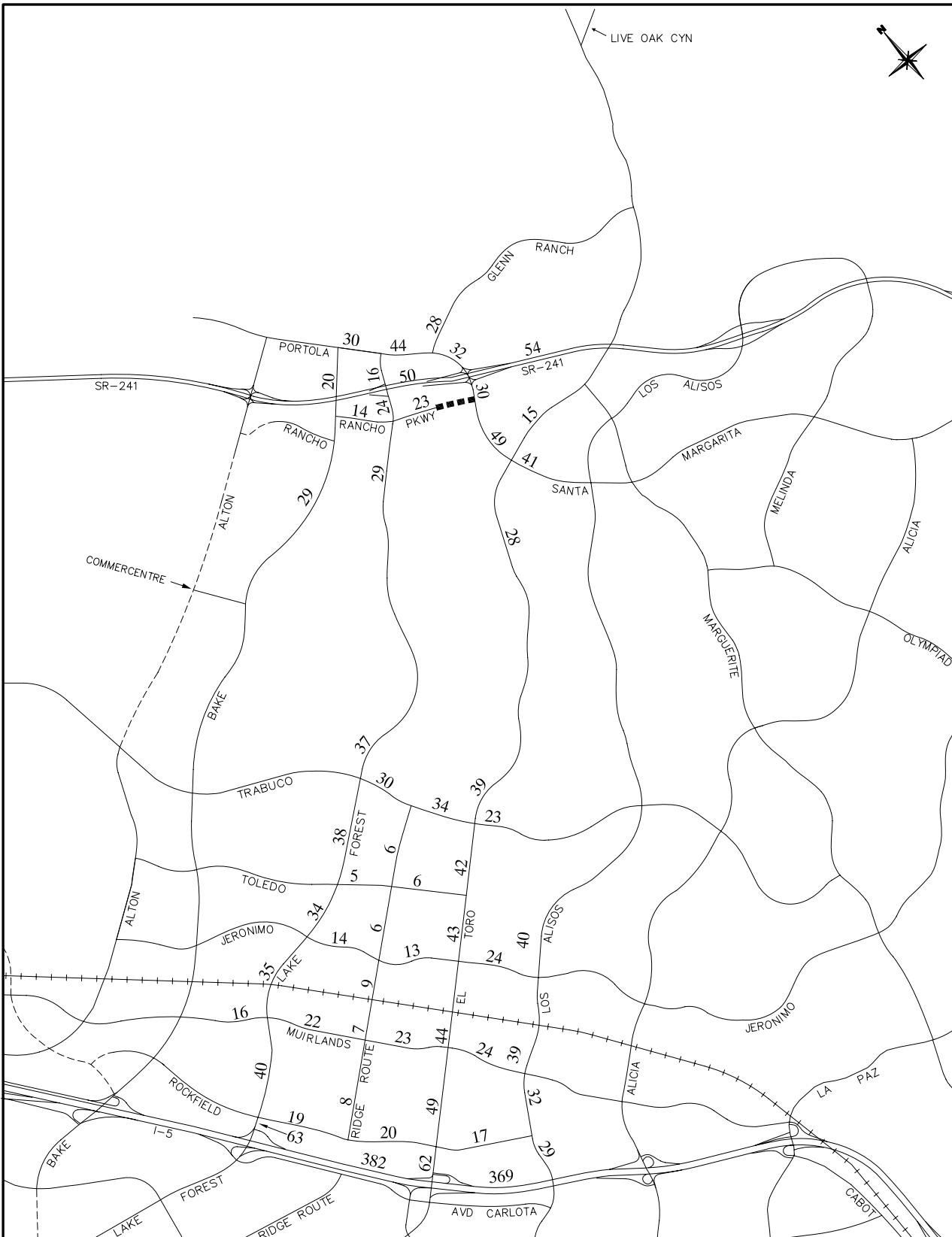


Legend

- Existing roadway
- - - Future roadway in by 2015

Figure 31

2015 ADT VOLUMES (000s)
- NO-PROJECT
(ALTERNATIVE 7)



Legend

- Existing roadway
- - - Future roadway in by 2015
- Future roadway in with-project only

Figure 32

2015 ADT VOLUMES (000s)
 - WITH-PROJECT
 (ALTERNATIVE 7)

relatively modest impacts on the surrounding street system during the AM and PM peak hours. The results of the analysis presented here indicate that the proposed project does not adversely impact any locations with the exception of Lake Forest Drive and Rancho Parkway in year 2015 cumulative in the PM peak hour. The improvements for this location are included in the LFTM Program and according to the analysis presented in this report their implementation would be required sooner than later. These improvements are needed as well as the Rancho Parkway extension to Portola Parkway and new intersection to accommodate the proposed project. The findings and conclusions mentioned here apply to either Alternative 7 or current General Plan conditions in the OSA.

Table 16

SHORT-TERM (YEAR 2015 CUMULATIVE) INTERSECTION LOS SUMMARY WITHIN STUDY AREA – ALTERNATIVE 7

Intersection	No-Project				With-Project				Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour			
	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM
2. Bake & Portola	.58	A	.81	D	.56	A	.81	D	-.02	.00
3. Lake Forest & Portola (a)	.61	B	.98	E	.54	A	.76	C	-.07	-.22
4. Glenn Ranch & Portola	.70	B	.67	B	.62	B	.64	B	-.08	-.03
5. Portola & SR-241 Ramps	.54	A	.68	B	.48	A	.61	B	-.06	-.07
7. Lake Forest & SR-241 NB	.38	A	.44	A	.31	A	.38	A	-.07	-.06
8. Lake Forest & SR-241 SB	.50	A	.52	A	.42	A	.44	A	-.08	-.08
9. Bake & Rancho North	.57	A	.75	C	.65	B	.75	C	.08	.00
10. Lake Forest & Rancho (a) (b)	.49	A	.67	B	.63	B	.92	E	.14	.25
11. Bake & Rancho South	.61	B	.66	B	.63	B	.70	B	.02	.04
12. El Toro & Portola/Santa Margarita	.64	B	.80	C	.68	B	.89	D	.04	.09
15. Lake Forest & Trabuco	.78	C	.86	D	.81	D	.84	D	.03	-.02
16. Ridge Route & Trabuco	.51	A	.65	B	.48	A	.63	B	-.03	-.02
17. El Toro & Trabuco	.71	C	.70	B	.66	B	.68	B	-.05	-.02
19. Lake Forest & Toledo	.48	A	.47	A	.47	A	.46	A	-.01	-.01
20. Ridge Route & Toledo	.31	A	.32	A	.30	A	.32	A	-.01	.00
21. El Toro & Toledo	.58	A	.58	A	.59	A	.56	A	.01	-.02
23. Lake Forest & Jeronimo	.67	B	.73	C	.66	B	.72	C	-.01	-.01
24. Ridge Route & Jeronimo	.44	A	.57	A	.43	A	.54	A	-.01	-.03
25. El Toro & Jeronimo	.77	C	.76	C	.79	C	.77	C	.02	.01
26. Los Alisos & Jeronimo	.78	C	.89	D	.78	C	.89	D	.00	.00
27. Lake Forest & Muirlands	.63	B	.86	D	.61	B	.86	D	-.02	.00
28. Ridge Route & Muirlands	.49	A	.66	B	.49	A	.67	B	.00	.01
29. El Toro & Muirlands	.65	B	.81	D	.66	B	.79	C	.01	-.02
30. Los Alisos & Muirlands (a)	.89	D	.92	E	.85	D	.91	E	-.04	-.01

Table 16 (cont.)

SHORT-TERM (YEAR 2015 CUMULATIVE) INTERSECTION LOS SUMMARY WITHIN STUDY AREA – ALTERNATIVE 7

Intersection	No-Project				With-Project				Difference	
	AM Peak Hour		PM Peak Hour		AM Peak Hour		PM Peak Hour			
	ICU	LOS	ICU	LOS	ICU	LOS	ICU	LOS	AM	PM
31. Lake Forest & Rockfield	.67	B	.75	C	.67	B	.76	C	.00	.01
32. Ridge Route & Rockfield	.42	A	.57	A	.43	A	.57	A	.01	.00
33. El Toro & Rockfield	.54	A	.65	B	.55	A	.65	B	.01	.00
34. Los Alisos & Rockfield	.81	D	.81	D	.82	D	.78	C	.01	-.03
35. Lake Forest & I-5 NB	.57	A	.64	B	.57	A	.64	B	.00	.00
36. Lake Forest & I-5/Carlota	.64	B	.81	D	.63	B	.82	D	-.01	.01
37. Paseo De Valencia & Carlota	.51	A	.77	C	.50	A	.76	C	-.01	-.01
38. El Toro & Bridger/I-5 NB	.65	B	.67	B	.65	B	.67	B	.00	.00
39. El Toro & Avd Carlota (a)	.60	A	1.00	E	.59	A	1.00	E	-.01	.00
40. Portola & Rancho	--	--	--	--	.52	A	.62	B	--	--

Abbreviations: ICU – intersection capacity utilization

LOS – level of service

NB – northbound

SB – southbound

- (a) This location is forecast to operate deficiently in the AM and/or PM peak hour under no-project and/or with-project conditions (i.e., the forecasted LOS is worse than the adopted LOS performance standard.
- (b) Significantly impacted by the proposed project according to the performance criteria.

REFERENCES

1. "City of Lake Forest Vacant Land Opportunities Phase III Traffic Study," Austin-Foust Associates, Inc., July 8, 2005.
2. "City of Lake Forest Vacant Land Opportunities Phase III Alternative 7 (Hybrid Alternative) Traffic Study," Austin-Foust Associates, Inc., November 7, 2007 (Approved by Lake Forest City Council on June 3, 2008).
3. "City of Lake Forest Vacant Land Opportunities Phase III Alternative 8 Traffic Study," Austin-Foust Associates, Inc., September 2009.

